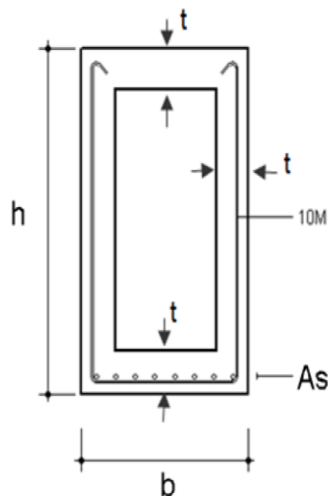




**CVG 3148 – REINFORCED CONCRETE DESIGN 2022
 ASSIGNMENT #3**

Problem #1



NOTE:

- Use: $f'_c = 30 \text{ MPa}$, $f_y = 400 \text{ MPa}$
- Steel reinforcement: $A_s = 10\text{-}20\text{M}$ bars
- Beam properties: $b = 400 \text{ mm}$, $h = 800 \text{ mm}$
- Clear cover = 40 mm (bottom, top and side cover), 10M stirrups
- Maximum aggregate size (a_{max}) = 20mm , interior exposure (**not exposed**)

Givens

f'_c	30 MPa
f'_y	400 MPa
a_{max}	20 mm
A_s	10-20M
Cover	40 mm
b	400 mm
h	800 mm
t	100 mm Sides, 200 mm for top and bottom

Part a)

Step 1)

Determine the effective distance of the Steel.

$$d = 740\text{mm}$$

Step 2)

Determine the parameters of the concrete compression block

$$\alpha_1 = 0.81$$

$$\beta_1 = 0.90$$

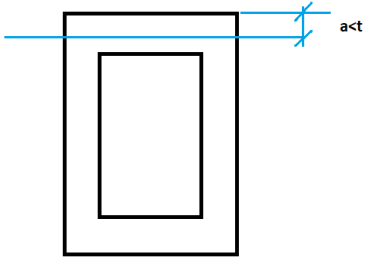
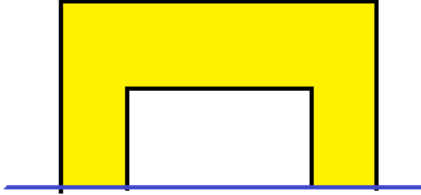
Step 3)

Determine the Area of Concrete Compression for the Steel Rebar.

$$A_{conc,comp} = 67591mm^2$$

Step 4)

Determine whether rectangular analysis governs.

check if $a_{conc} < t_{top}$ governs	check if $a_{con} > t_{top}$ governs
 <p style="text-align: center;"> $A_{conc,comp} = 400 * a$ $a = 169 < 200 \text{ o.k}$ </p> <p style="text-align: center;"> $\bar{a} = \frac{169}{2} = 84.5$ </p>	 <p style="text-align: center;"> $A_{conc,comp} = 400 * 200 + 2 * 100 * (a - 100)$ </p> <p style="text-align: center;"> $A=38 < 200$ does not make sense </p>

Step 5)

Determine distance between centroid of force blocks.

$$Z = d - \bar{a}$$

$$= 655.5mm$$

Step 6)

Determine the Moment resistance (concrete or Steel)

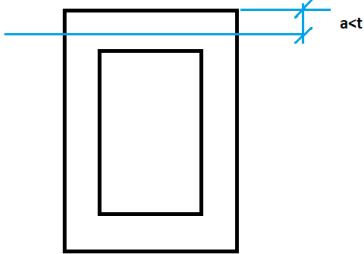
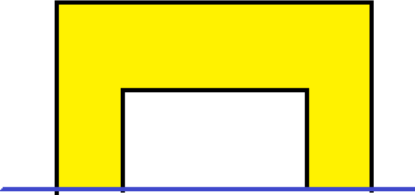
$$Mr = \phi_s \cdot A_s \cdot f_y \cdot z$$

$$= 699.8 \text{ kN.m}$$

Part b)

Step 1)

Determine whether rectangular analysis governs.

check if $a_{conc} < t_{top}$ governs	check if $a_{con} > t_{top}$ governs
 <p> $A_{conc,comp} = 400 * a$ $a = 169 > 100$ NO </p>	 <p> $A_{conc,comp} = 400 * 100 + 2 * 100 * (a - 100)$ $a = 238 > 100$ OK $\bar{a} = 78.2mm$ </p>

Step 2)

Determine distance between centroid of force blocks.

$$Z = d - \bar{a}$$

$$= 661.8mm$$

Step 3)

Determine the Moment resistance (concrete or Steel)

$$Mr = \phi_s \cdot A_s \cdot f_y \cdot z$$

$$= 706.5 \text{ kN.m}$$

Part c)

Step 1)

Check Yielding for Beam a:

$$\left(\frac{c}{d}\right) < \left(\frac{c}{d}\right)_{max}$$

$$0.25 < 0.64 \text{ OK}$$

Check Yielding for Beam b:

$$\left(\frac{c}{d}\right) < \left(\frac{c}{d}\right)_{max}$$

$$0.36 < 0.64 \text{ OK}$$

Part d)**Step 1)**

Check Steel Minimum and Spacing requirements Check $A_s \geq A_{smin}$

$$A_{smin} = \frac{0.2\sqrt{f'_c}}{f_y}bh$$

$$= 876.4 \text{ mm}^2$$

$$A_s \geq A_{smin}, 3140 > 876.4 \text{ mm}^2 \text{ OK}$$

Step 2)

Check Spacing and Cover Requirements

$$spacing = \frac{b - 2(\text{clear cover}) - 2(d_{stirrup}) - n(d_{bar})}{n_{bars} - 1}$$

$$s_{min} \geq \max(1.4 d_{bar}, 1.4 d_{agg}, 30\text{mm})$$

$$= \max(28\text{mm}, 28\text{mm}, \mathbf{30\text{mm}})$$

$$s = 11.1 < 30\text{mm} \rightarrow \mathbf{NOT OK}$$

Step 3)

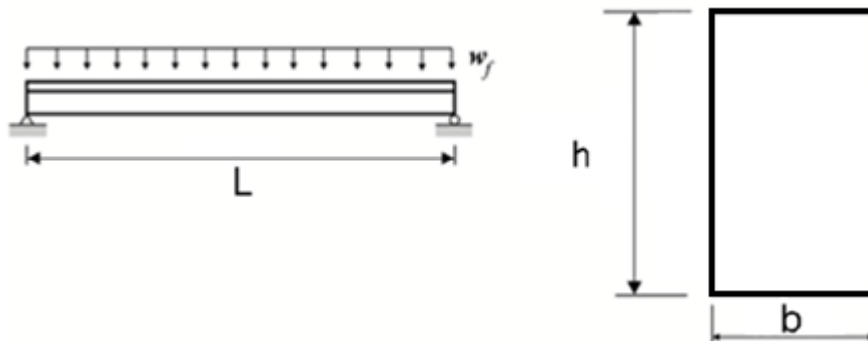
Check crack control

$$z = f_s(d_c \times A)^{1/3} = 240 \times (60 \times 4800)^{1/3}$$

$$= 15849 \text{ N/mm} < 30000 \text{ N/mm (interior exposure)}$$

OK for interior exposure

Problem #2



NOTE:

- Beam dimensions are fixed: $b = 350 \text{ mm}$, $h = 700 \text{ mm}$
- Beam span, $L = 6 \text{ m}$
- Use: $f'_c = 30 \text{ MPa}$, **20M bars** and **10M stirrups**
- Density of concrete for self-weight calculation: 24 KN/m^3
- Clear cover = 30 mm (bottom and side cover)
- Maximum aggregate size (a_{max}) = 20 mm
- Start by trying 1 layer (row) of rebar, if it does not work try 2 layers

Part a)

Step 1)

$$SW(\text{Self - Weight}) = 5.88 \frac{kN}{m}$$

$$DL = 13.88 \frac{kN}{m}$$

$$Wf = 84.85 \frac{kN}{m}$$

$$Mf = 381.8 \text{ kN.m}$$

$$Mf = Mr$$

Step 2)

Determine the parameters of the concrete compression block

$$\alpha_1 = 0.81$$

$$\beta_1 = 0.90$$

Step 3)

Determine estimated depth for 1 Layer of Reinforcement

$$d_a = h - 70mm$$

$$= 630mm$$

OR

$$d_b = h - cover - d_{tie} - \frac{d_{rebar}}{2}$$

$$= 650mm$$

Step 4)1st iteration

Estimate a;

$$a = 0.3 * d_a$$

$$= 189 mm (195mm)$$

Determine A_s based on M_r ;

$$M_r = \phi_s * A_s * f_y * \left(d - \frac{a}{2}\right)$$

$$A_{s1} = \frac{M_r}{\phi_s * f_y * \left(d - \frac{a}{2}\right)}$$

$$= 2097 mm^2 (2032.5 mm^2)$$

Determine revised a;

$$a_1 = \frac{\phi_s * A_s * f_y}{\alpha_1 * \phi_c * f_c * b}$$

$$= 129mm$$

$$a_1 > 5\% \text{ difference}$$

Step 5)2nd iteration 568.95

$$a_1 = 129mm$$

$$A_{s2} = 1985.7mm^2$$

$$a_2 = 122.1mm$$

$$a_2 > 5\% \text{ difference}$$

Step 6)3rd iteration

$$a_2 = 122.1mm$$

$$A_{s2} = 1973.7mm^2$$

$$a_3 = 121.4mm$$

$$a_3 < 5\% \text{ difference! OK}$$

Step 7)

Design

$$d_{bar} = 20mm, a_{bar} = 320mm^2, cover = 30mm$$

$$\#bar = A_{s2}/a_{bar}$$

$$= 6,17 \text{ bars}$$

$$= \text{round to 7 bars}$$

$$a_{revised} = \frac{\phi_s \cdot A_s \cdot f_y}{\alpha 1 \cdot \phi_c \cdot f_c \cdot b}$$

$$= 137.77mm$$

$$M_r = \phi_s \cdot A_s \cdot f_y \cdot \left(d - \frac{a_{revised}}{2} \right)$$

$$= 427.35 \text{ KN} \cdot m > M_f, \text{ O.K}$$

Step 8)

Check Steel Minimum and Spacing requirements Check $A_s \geq A_{smin}$

$$A_{smin} = \frac{0.2\sqrt{f'_c}}{f_y} bh$$

$$= 670.96 \text{ mm}^2$$

$$A_s \geq A_{smin}, 2240 > 670.96 \text{ mm}^2 \text{ OK}$$

Step 9)

Check Spacing and Cover Requirements

$$spacing = \frac{b - 2(\text{clear cover}) - 2(d_{stirrup}) - n(d_{bar})}{n_{bars} - 1}$$

$$s_{min} \geq \max(1.4 d_{bar}, 1.4 d_{agg}, 30mm)$$

$$= \max(28mm, 28mm, \mathbf{30mm})$$

$$s = 21.67 > 30mm \rightarrow \mathbf{NOT OK}$$

Bars are placed in two layers as 5 + 2 (total 7) or as 4 + 4 (total 8)

Step 10)

Check for steel yielding

$$\left(\frac{c}{d}\right) < \left(\frac{c}{d}\right)_{max}$$

$$0.24 < 0.64 \text{ OK}$$

Part b)**Step 1)**

Setup Quadratic

$$0 = \left[\frac{\phi_s f_y^2}{2 * \alpha_1 * \phi_c * f_c * b} \right] * A_s^2 - [\phi_s * f_y * d] * A_s + M_r$$

$$A = 10.46$$

$$B = -214200$$

$$C = M_r$$

$$= 381.8 \text{ Kn} * \text{m} * (1e6)$$

$$A_s = 1972.4 \text{ mm}^2$$

$$d_{bar} = 20 \text{ mm}, a_{bar} = 320 \text{ mm}^2, \text{ cover} = 30 \text{ mm}$$

$$\#bar = 6,16 \text{ bars}$$

$$= \text{round to 7 bars}$$

Part c)**Step 1)**

$$M_r = K_r * b * d^2$$

$$K_r = 2.75 \text{ MPa}$$

Check Table Table 2-1 @ $f_c = 30 \text{ MPa}$

$$K_r = 2.7 \text{ MPa} \rightarrow \rho = 0.88 \%$$

$$K_r = 2.8 \text{ MPa} \rightarrow \rho = 0.91 \%$$

$$\text{Interpolation} \rightarrow \rho = \frac{(0.91\% - 0.88\%)}{(2.8 - 2.7)} * (2.75 - 2.7) + 0.88\% = 0.895\%$$

$$A_s = \rho * b * d = 1973.5 \text{ mm}^2$$

$$\#bar = 6.17$$

$$= \text{round to 7 bars}$$

No need to check spacing and minimum reinforcement. Same # of bars.