



Civil Engineering Department

October 2nd, 2019

CVG2132 – FUNDAMENTALS OF ENVIRONMENTAL ENGINEERING

Homework #3

Professor: Chris Kinsley

Due Date: Oct. 11th, 2019 (3:30pm) – Dropbox « CVG 2132 », Mezzanine A (0.5) CBY

- 1) Compute the following –
 - a) Derive the expressions for concentration exiting a CSTR and a PFR for a zero-order reaction. Which reactor design is more efficient in terms of volume and hydraulic retention times? Why? State assumptions.
 - b) A chemical element degrades according to first order reaction. Its half-life is 25 days, determine rate constant, k. How long does it take for the element to degrade by 80%? Does $t_{1/2}$ depend on initial concentration?

Solution:

a)

For CSTR –

Assumptions

- steady state,
- $A \rightarrow B$,
- Influent and effluent are dilute.

From Lecture 6, slide 9, for a single reactant to single product irreversible reaction rate equation can be given by -

$$r = -\frac{dC}{dt} = kC^0 \quad \text{or} \quad \frac{dC}{dt} = -k$$

Performing mass balance for the constituent of interest in the CSTR

$$Accumulation = Input - Output + Generation$$

Since its in steady state, accumulation = 0

$$\left(\frac{dC}{dt}\right)_{\text{accum}} V = QC_0 - QC_t + \left(\frac{dC}{dt}\right)V$$

$$QC_0 = QC_t + kV$$

$$Q(C_0 - C_t) = kV$$

$$\frac{V}{Q} = \tau = \frac{C_0 - C_t}{k}$$

$$C_t = C_0 - k\tau$$

For PFR –

Assumptions

- Steady state,
- $A \rightarrow B$,
- Influent and effluent are dilute ($\rho_x = \rho_{x+\Delta x} = \rho_w$)

Performing a mass balance for the constituent of interest on the differential volume element of the PFR

$$\text{Accumulation} = \text{Input} - \text{Output} + \text{Generation}$$

Since its in steady state, accumulation = 0

$$0 = Q_x C_x - Q_{x+\Delta x} C_{x+\Delta x} + rV$$

$$(Q_{x+\Delta x} C_{x+\Delta x} - Q_x C_x) / V = r$$

Since the influent and effluent are dilute, $Q_{x+\Delta x} = Q_x = Q$

$$C_{x+\Delta x} - C_x = \Delta C$$

From diagram: $\Delta V = \Delta X * A$ where A is the cross-sectional area.

$$\frac{Q \Delta C}{A \Delta X} = r$$

From Lecture 6, slide 9, for a single reactant to single product irreversible reaction rate equation can be given by -

$$r = -\frac{dC}{dt} = kC^0 \quad \text{or} \quad \frac{dC}{dt} = -k$$

Performing mass balance for the constituent of interest in the PFR

$$\text{Accumulation} = \text{Input} - \text{Output} + \text{Generation}$$

Since its in steady state, accumulation = 0

Also,

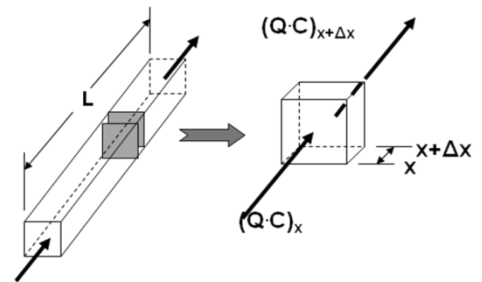
$$\frac{Q \Delta C}{A \Delta X} = -k$$

$$\frac{Q}{A} \int_{C_0}^{C_t} dC = -K \int_0^L dX$$

$$Q(C_t - C_0) = -k(L)A$$

$$C_0 - C_t = k\tau$$

$$C_t = C_0 - k\tau$$



As the design equations are the same for a zero-order removal, both of the reactors will perform with the same efficiency in terms of volume and hydraulic retention time.

b) According to lecture 6, slide 37,

$$\text{first-order: } C = C_0 e^{-kt} \rightarrow 0.5C_0 = C_0 e^{-kt_{1/2}} \rightarrow \ln 0.5 = -kt_{1/2} \rightarrow t_{1/2} = -\frac{\ln 0.5}{k} = \frac{0.693}{k}$$

$$t_{1/2} = 25 \text{ days} = \frac{0.693}{k}$$

$$k = 0.02772 \text{ d}^{-1}$$

If 80% of the element degrades, 20% of the element still remains.

$$\frac{C}{C_0} = e^{-kt}$$

$$\frac{C}{C_0} = 0.2 = e^{-kt}$$

$$\ln\left(\frac{C}{C_0}\right) = -kt$$

$$\ln(0.2) = -0.02772 \text{ d}^{-1} \times t$$

$$t = 58 \text{ days}$$

- 2) Phosphorous is considered as one of the limiting factors of eutrophication in lakes and rivers. The goal of a pilot plant study is to treat phosphorous from influent wastewater. The process involves a CSTR that removes phosphorus according to second order kinetics with a k value of $5.2 \frac{L}{mg \cdot d}$. The CSTR has a flow rate of 520 L/d, a volume of 90 L and an initial concentration of 3.5 mg P/L.
- Derive the HRT equation for the second order CSTR
 - If the effluent P concentration to prevent eutrophication is 0.03 mg P/L, do the current parameters meet this goal?
 - If the flow rate were to be decreased by 30% and volume of CSTR increased to 130L, what is the new effluent P concentration?

Solution –

Assumptions

- As there is no indication of change with time, steady state conditions apply
- Unless stated otherwise. we can assume dilute streams.

Knowns and Unknowns

Known: $Q = 520 \text{ L/d}$, $V = 90 \text{ L}$, $C_0 = 3.5 \text{ mg P/L}$, $k = 5.2 \frac{L}{mg \cdot d}$

Unknown: HRT, C and New C

Determining the HRT equation for the second order CSTR

Performing a constituent mass balance around the system

$$\text{Accumulation} = \text{Input} - \text{Output} - \text{Reaction}$$

Accumulation = 0 due to steady state

$$0 = QC_0 - QC - rV_{CSTR}$$

$$0 = QC_0 - QC - kC^2V_{CSTR}$$

Divide by Q

$$0 = C_0 - C - kC^2\tau$$

$$\tau = \frac{(C_0 - C)}{C^2k}$$

$$\tau = \frac{\left(\frac{C_0}{C} - 1\right)}{kC}$$

Rearrange to form -

$$0 = kC^2\tau + C - C_0$$

a) We know that,

$$\tau = \frac{V}{Q} = \frac{90L}{520 L/d} = \mathbf{0.173 \text{ day}}$$

b) Substitute the value of HRT obtained in the derived equation above,

$$0 = kC^2\tau + C - C_0$$

$$0 = \left(5.2 \frac{L}{mg \cdot d} \times C^2 \times 0.173 \text{ day}\right) + C - 3.5 \frac{mg P}{L}$$

$$0 = 0.8996C^2 + C - 3.5$$

On solving the above quadratic equation, we get,

$$\mathbf{C = 1.49 \frac{mg P}{L}}$$

This effluent does not meet the requirements to prevent eutrophication.

c) Flowrate reduced by 30%, new volume of CSTR is 130L

$$\text{New } Q = 520 \frac{L}{d} - \left(0.3 \times 520 \frac{L}{d}\right) = 364 \frac{L}{d}$$

$$\text{New } \tau = \frac{V}{Q} = \frac{130L}{364 L/d} = 0.357 \text{ day}$$

$$0 = kC^2\tau + C - C_0$$

$$0 = \left(5.2 \frac{L}{mg \cdot d} \times C^2 \times 0.357 \text{ day}\right) + C - 3.5 \frac{mg P}{L}$$

We get, new effluent concentration, $C = 1.129 \frac{mg P}{L}$

- 3) A CSTR reactor is used to treat an industrial waste using a reaction which destroys the waste according to first order kinetics, where $k = 0.226 \text{ day}^{-1}$. The reactor volume is 450 m^3 , volumetric flow rate of the single inlet and exit is $55 \text{ m}^3/\text{day}$, and the inlet waste concentration is 100 mg/L .
- Derive the design equation.
 - What is the outlet concentration?
 - Determine the volume required for a PFR to obtain the same degree of pollutant reduction as the CSTR. Assume the flow rate and k are unchanged.
 - Which system is more efficient (% removal / m^3 of volume)?

Solution-

Assumptions –

- As there is no indication of change with time, steady state conditions apply
- Unless stated otherwise, we can assume dilute streams.

a) Derivation of design equation

The simplified first-order removal reaction:

$$r = -\frac{dC}{dt} = kC^1 \quad \text{or} \quad \frac{dC}{dt} = -kC$$

Performing a mass balance for the constituent of interest on the CSTR reactor –

$$\text{Accumulation} = \text{Input} - \text{Output} - \text{Reaction}$$

$$\left(\frac{dC}{dt}\right)_{\text{accum}} V = QC_0 - QC_t + \left(\frac{dC}{dt}\right)V$$

Steady state = No accumulation

$$0 = QC_0 - QC_t - kC_t V$$

$$kC_t V = Q(C_0 - C_t)$$

$$\text{HRT} = \frac{V}{Q} = \frac{(C_0 - C_t)}{kC_t} = \frac{(C_0/C_t - 1)}{k}$$

b) Determine the HRT and C

$$HRT = \frac{V}{Q} = \frac{450 \text{ m}^3}{55 \text{ m}^3/\text{d}} = 8.18 \text{ d}$$

$$HRT \times k = \frac{C_0}{C} - 1$$

$$C = \frac{C_0}{(HRT \times k) + 1} = \frac{100 \text{ mg/L}}{(8.18 \text{ d} \times 0.226 \text{ d}^{-1}) + 1} = 35.1 \text{ mg/L}$$

c) The CSTR achieved a pollutant decrease of $\frac{C}{C_0} = \frac{35.1 \text{ mg/L}}{100 \text{ mg/L}} = 0.351$

Derive equation for PFR

$$\text{Accumulation} = \text{Input} - \text{Output} + \text{Generation}$$

Since its in steady state, accumulation = 0

$$0 = Q_x C_x - Q_{x+\Delta x} C_{x+\Delta x} + rV$$

$$(Q_{x+\Delta x} C_{x+\Delta x} - Q_x C_x) / V = r$$

Since the influent and effluent are dilute, $Q_{x+\Delta x} = Q_x = Q$

$$C_{x+\Delta x} - C_x = \Delta C$$

From diagram: $\Delta V = \Delta X * A$ where A is the cross-sectional area.

$$\frac{Q \Delta C}{A \Delta X} = r$$

The simplified first-order removal reaction:

$$r = -\frac{dC}{dt} = kC^1 \quad \text{or} \quad \frac{dC}{dt} = -kC$$

$$\frac{Q \Delta C}{A \Delta X} = -kC$$

$$\frac{Q}{A} \int_{C_0}^{C_t} \frac{dC}{C} = -k \int_0^L dX$$

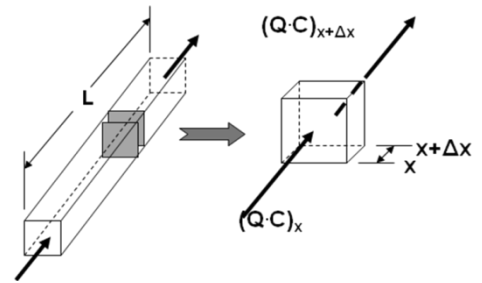
$$\ln\left(\frac{C_t}{C_0}\right) = \frac{-kAL}{Q}$$

$$\ln\left(\frac{C_t}{C_0}\right) = \frac{-kV}{Q}$$

$$\ln(0.351) = \frac{-0.226 \text{ d}^{-1} \times V}{55 \text{ m}^3/\text{d}}$$

$$-1.046 = -0.004 \times V$$

$$V = 261.75 \text{ m}^3$$



d) Volumetric efficiency of CSTR

$$Efficiency = \frac{C_0 - C}{Volume\ of\ CSTR} = \frac{(100 - 35.1) \times \frac{mg}{L}}{450m^3} = 0.144 \frac{\frac{mg}{L}}{m^3}$$

Volumetric efficiency of PFR

$$Efficiency = \frac{C_0 - C}{Volume\ of\ PFR} = \frac{(100 - 35.1) \times \frac{mg}{L}}{261.75m^3} = 0.248 \frac{\frac{mg}{L}}{m^3}$$

Note - As expected, PFR volume is smaller than the CSTR used initially. Hence, the same trend follows for volumetric flowrate.

- 4) The Ottawa wastewater treatment facility is investigating the potential of ammonia treatment at their facility. The aeration basin of the Ottawa facility is a baffled tank that can be modelled as an ideal PFR. The wastewater facility's influent flow rate is 634 000 m³/d with an assumed influent ammonia concentration of 25 mg NH₃-N/L. The baffled tank has a volume of 23 000 m³ and an ammonia removal reaction kinetics constant of $8.7 \frac{L}{mg \cdot d}$. Note that the ammonia removal reaction is dependent on both the ammonia concentration and dissolved oxygen concentration, $r = -kC_{NH_3-N}C_{O_2}$. The Ottawa wastewater treatment facility is considering providing a constant dissolved oxygen concentration of 4 mg O₂/L or 10 mg O₂/L in the aeration basin to attain an effluent ammonia target of 1.25 mg NH₃-N/L.
- Derive the HRT equation for the baffled tank (state all assumptions and show all work).
 - Determine which of the oxygen concentrations (4 mg O₂/L or 10 mg O₂/L) are required to meet the effluent ammonia requirement. As all options must be explored, please calculate both options.

a) Assumptions

- As there is no indication of change with respect to time, we can assume steady state applies
- Unless stated otherwise, assume dilute streams. Hence, the density of all streams is equivalent to the density of water i.e $\rho_w = \rho = 1000\ kg/m^3$

Based on the units of the k rate constant, we can assume that it is a pseudo first order reaction. Derive the pseudo-first order PFR equation

The problem statement describes that the concentration of oxygen therefore we can combine the C_{O2} and k₂ terms to get a first order k' term. Therefore $r = -k'C_{NH_3-N} = -k_2C_{NH_3-N}C_{O_2}$

Accumulation = In – Out ± Generation

Accumulation = 0, due to steady state

As there is only a single stream entering and leaving the reactor and steady state conditions apply.

$$Q_{in} = Q_{eff} = Q$$

$$0 = QC_0 - QC_{eff} + rV$$

As this is a PFR set $C_0 = C_x$ and C_{eff} as $C_{x+\Delta x}$

$$0 = QC_x - QC_{x+\Delta x} + rV$$

$$r = \frac{Q(C_{x+\Delta x} - C_x)}{V}$$

$$V = \Delta V = A\Delta X$$

$$C_{x+\Delta x} - C_x = \Delta C$$

$$r = \frac{(Q\Delta C)}{(A\Delta X)}$$

$$r = -kC$$

$$-kC = \frac{(Q\Delta C)}{(A\Delta X)}$$

$$-\frac{A\Delta X}{Q} = \frac{\Delta C}{kC}$$

Integrate the length of the reactor

$$-\frac{A}{Q} \int_0^L dX = \frac{1}{k} \int_{C_0}^C \frac{1}{C} dC$$

$$-\tau = \frac{(\ln C - \ln C_0)}{k}$$

$$\tau = \frac{(\ln C_0 - \ln C)}{k}$$

Given Information

$$Q_{in} = 634\,000 \text{ m}^3/\text{d}$$

$$V_{PFR} = 23\,000 \text{ m}^3$$

$$K_2 = 8.7 \frac{\text{L}}{\text{mg}\cdot\text{d}} = 8.7 \frac{\text{m}^3}{\text{g}\cdot\text{d}}$$

$$DO_1 = 4 \text{ g O}_2/\text{m}^3$$

$$DO_2 = 10 \text{ g O}_2/\text{m}^3$$

$$C_{in} = 25 \text{ mg NH}_3\text{-N/L} = 25 \text{ g N/m}^3$$

Unknowns: C_{eff} at 4 g O₂/L and C_{eff} at 10 g O₂/L

The system diagram can be shown as below

Determine the effluent concentration with a DO of 4 g O₂/m³

Using the equation previously defined, we get

$$T = (\ln C_0 - \ln C)/k$$

Where $C_0 = C_{in}$, $C = C_{eff}$, $\tau = V_{PFR}/Q$ and $k = k' = k_2 C_{O_2}$

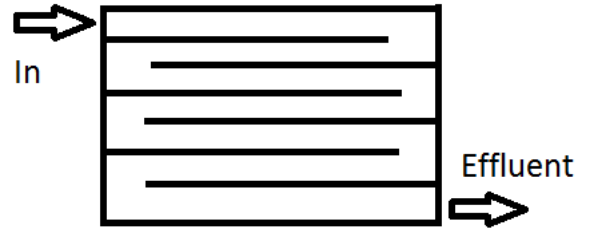
$$\text{For } C_{O_2} = 4 \text{ g O}_2/\text{m}^3; k' = 8.7 \frac{\text{m}^3}{\text{g} \times \text{d}} \times 4 \frac{\text{g O}_2}{\text{m}^3} = 34.8 \frac{1}{\text{d}}$$

Rearranging the equation,

$$C_{eff} = C_{in} \times e^{-\tau k'}$$

$$\tau = \frac{\text{Volume of PFR}}{Q} = \frac{23000 \text{ m}^3}{634000 \text{ m}^3/\text{d}} = 0.036 \text{ day}$$

$$C_{eff} = \left(25 \frac{\text{g}}{\text{m}^3}\right) \times e^{-(0.036 \text{ day} \times 34.8 \frac{1}{\text{d}})} = 7.14 \frac{\text{g}}{\text{m}^3}$$



Determine the effluent concentration with a DO of 10 g O₂/m³

Using the equation previously defined, we get

$$\tau = (\ln C_0 - \ln C)/k$$

Where $C_0 = C_{in}$, $C = C_{eff}$, $\tau = V_{PFR}/Q$ and $k = k' = k_2 C_{O_2}$

$$\text{For } C_{O_2} = 10 \text{ g O}_2/\text{m}^3; k' = 8.7 \frac{\text{m}^3}{\text{g} \times \text{d}} \times 10 \frac{\text{g O}_2}{\text{m}^3} = 80 \frac{1}{\text{d}}$$

Rearranging the equation,

$$C_{eff} = C_{in} \times e^{-\tau k'}$$

$$\tau = \frac{\text{Volume of PFR}}{Q} = \frac{23000 \text{ m}^3}{634000 \text{ m}^3/\text{d}} = 0.036 \text{ day}$$

$$C_{eff} = \left(25 \frac{\text{g}}{\text{m}^3}\right) \times e^{-(0.036 \text{ day} \times 80 \frac{1}{\text{d}})} = 1.09 \frac{\text{g}}{\text{m}^3}$$

Therefore, a dissolved oxygen concentration of 10 g O₂/m³ is required to attain a final effluent concentration lower than 1.25 mg NH₄⁺-N/L.