

Lecture 4.1: Virtual Work - Trusses

Lecture outline:

1. Energy methods for computing deflections
2. Principal of virtual work
3. Virtual work: trusses
4. Procedure

1. Energy methods for computing deflections:

Energy methods are based on the principal of conservation of energy:

$$U_e = U_i$$

work done by all external forces acting on a structure, U_e

= internal work (strain energy stored in the structure), U_i

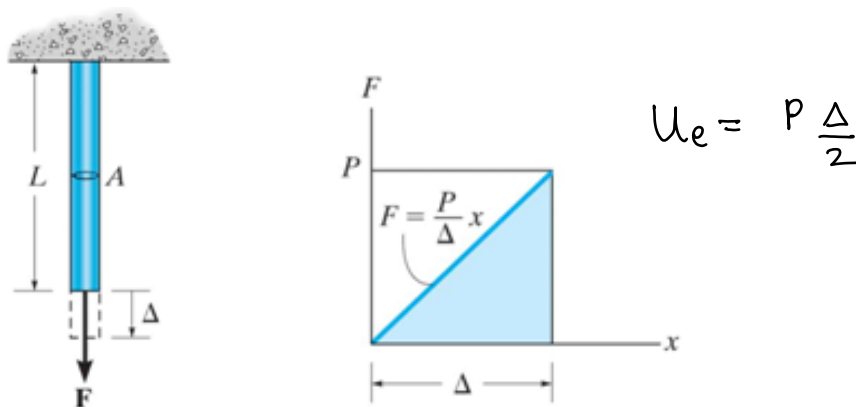
External work due to a Force (U_e)

When a force, F , displaces an amount dx in the same direction as the force the total work done is:

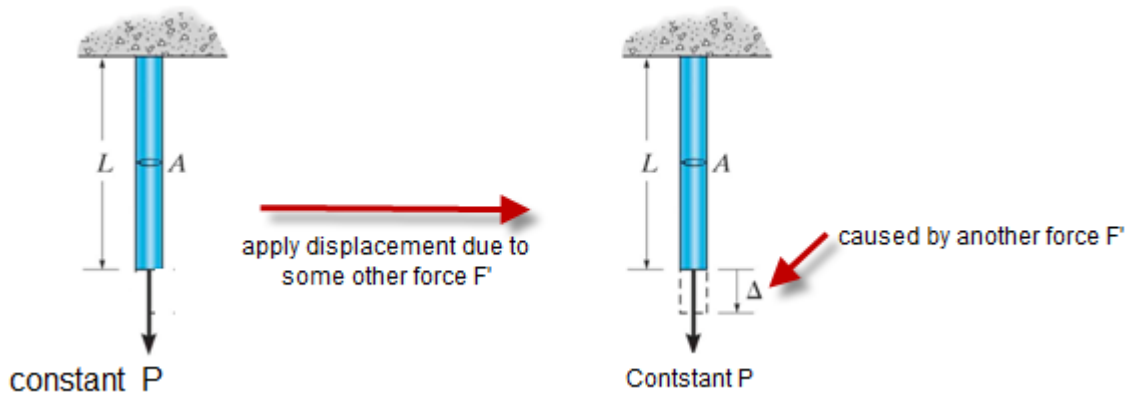
$$dU_e = Fdx \rightarrow U_e = \int_0^x Fdx$$

Work = Area under the force-displacement diagram

If the force is gradually applied to a linearly elastic structure we may express the variation of force with displacement as



Another case that we are interested in is the work done by a constant force while its point of application undergoes a displacement caused by some other force independent of P:



Internal work (U_i)

Strain energy due to an axial force: When an axial force, N , is applied to a linear elastic bar, the external work done by the bar will be converted into strain energy.

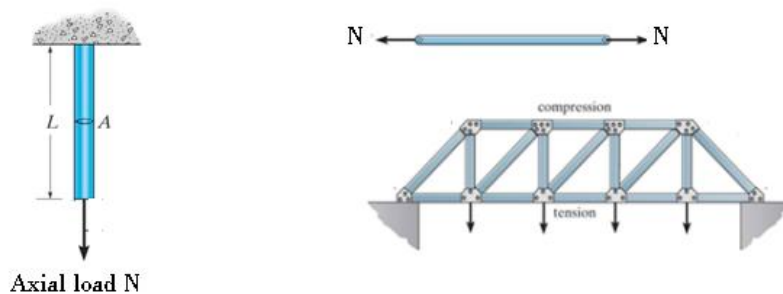
The **stress** in the bar is given by: $\sigma = \frac{N}{A}$ Where A is the cross-sectional area of the bar.

The **final strain** of the bar is: $\varepsilon = \frac{\delta}{L}$ Where ε is the strain and δ is the final displacement.

If the bar is linear elastic then Hooke's law will be valid: $\sigma = E\varepsilon$

Solving for the internal displacement: $\delta = \frac{NL}{AE}$

Substituting into the equation for work: $U_i = \frac{1}{2}N\delta = \frac{1}{2}N\frac{NL}{AE}$



2. Principle of virtual work

The principle of virtual work is based on the conservation of energy and was developed by John Bernoulli in 1717 (also called **unit load method**)

It is a general procedure for determining the displacement and slope at any point in a structure (trusses, beams and frames).

If we apply a series of external forces P to a structure:

- it will cause a set of internal forces, N , through the structure.

Due to the loading:

- EXTERNAL displacements Δ will occur due to EXTERNAL loads P
- INTERNAL displacements δ will occur due to INTERNAL loads N

$$\sum(P * \Delta) = \sum(N * \delta)$$

Work of External loads = Work of Internal loads

As luck would have it:

- point where we typically want to find $\Delta \neq$ location of external loads P (usually)

In order to calculate displacement anywhere in a structure we can use the concept of a “**virtual**” load. “**Virtual**” means that the load is imaginary and does not exist as part of the real loading system.

Place P' in the direction of the unknown displacement

P' causes a set of internal virtual forces $\rightarrow n$

Next the structure is subjected to the real loading (P):

- causes real displacement Δ
- causes real internal forces $N \rightarrow$ real internal displacements δ
- The internal virtual loads n move through the real internal displacements δ

The external work must be equal to the internal work so we can write:

$$P' \cdot \Delta = \sum n \cdot \delta$$

By choosing $P' = 1$ we end up with:

$$1 \cdot \Delta = \sum n \cdot \delta$$

3. Virtual work method: Trusses

We can apply the above equation for solving the displacement in a truss.

If the applied load causes linear elastic deformations, the **internal** displacements $\rightarrow \delta = \frac{NL}{AE}$

Therefore the virtual force equation $1 \cdot \Delta = \sum n \cdot \delta$ becomes:

$$1 \cdot \Delta = \sum n \cdot \frac{NL}{AE}$$

- 1 = external **virtual** unit load
- n = internal force in truss member caused by the external **virtual** force
- Δ = external displacement due to **real** loads
- N = internal force in truss member caused by the external **real** loads
- L, A, E = length, cross-section area & modulus of elasticity of a member

Temperature effects:

In some cases trusses members may change their length due to temperature:

- α is the coefficient of thermal expansion for a member
- ΔT is the change in temperature
- $\Delta L = \alpha \Delta T L$ is the resulting change in member length

To determine the displacement at a truss joint due to this temperature change we use:

$$1 \cdot \Delta = \sum n \cdot \alpha \Delta T L$$

- if a member undergoes an **increase** in temperature, ΔT will be **positive (+)**
- if a member undergoes an **decrease** in temperature, ΔT will be **negative (-)**

Fabrication errors:

In some cases trusses members may be made longer or shorter either due to fabrication errors:

- ΔL is the resulting change in member length

If a truss member is shorter or longer than expected the resulting displacement:

$$1 \cdot \Delta = \sum n \cdot \Delta L$$

- if a fabrication error **increases** the length of a member: ΔL will be **positive (+)**
- if a fabrication error **decreases** the length of a member: ΔL will be **negative (-)**

In a problem with all these effects $\rightarrow 1 \cdot \Delta = \sum n \cdot \frac{NL}{AE} + \sum n \cdot \alpha \Delta T L + \sum n \cdot \Delta L$

4. Procedure

Procedure - VW for trusses:

1. Ensure that truss is determinate and stable:
 - Recall if: $b + r = 2j \rightarrow$ truss is determinate
2. Structure subjected to real forces:
 - Use method of sections/joints to determine internal forces due to **real** loads:
 - get “N” values
3. Structure subjected to virtual force:
 - Remove real loads
 - Place a unit load (= 1 kN) on the truss at the joint where the displacement is to be determined
 - Load should be in same direction as the displacement
 - Use method of sections/joints to determine internal forces due to virtual loads
 - get “n” values
 - assume that T = positive, C = negative
4. Apply virtual-work equation:

$$1 \cdot \Delta = \sum n \cdot \frac{NL}{AE} + \sum n \cdot \alpha \Delta T L + \sum n \cdot \Delta L$$

- if a member undergoes an **increase** in temperature, ΔT will be **positive (+)**
- if a fabrication error **increases** the length of a member: ΔL will be **positive (+)**

General notes:

- Be careful to retain the algebraic sign for each of “n” and “N”
 - If right hand-side is positive (+) : displacement is in same direction as unit load
 - If left hand-side is negative (-) : displacement is in opposite direction as unit load
- Be careful to be consistent with units
 - GPa = 1×10^6 kN/m²
 - mm² = 1×10^{-6} m²

Lecture 4.2: Virtual work - Beams

Lecture outline:

1. Principal of virtual-work for beams/frames
2. Procedure
 - Option 1: Direct Integration
 - Option 2A: Evaluating $\int_b^a m M dx$ using graphical method
 - Option 2B: Evaluating $\int_b^a m M dx$ using Mohr's integration tables
3. Integration by parts
4. Other effects: axial, shear, torsion and temperature effects

1. Principal of virtual work: beams/frames

The principal of virtual work we discussed for trusses can also be used to find the deflection of beams. Deflection in a beam is primarily due to strain caused by bending.

Real loads \rightarrow Real internal moment, $M \rightarrow$ real displacement Δ .

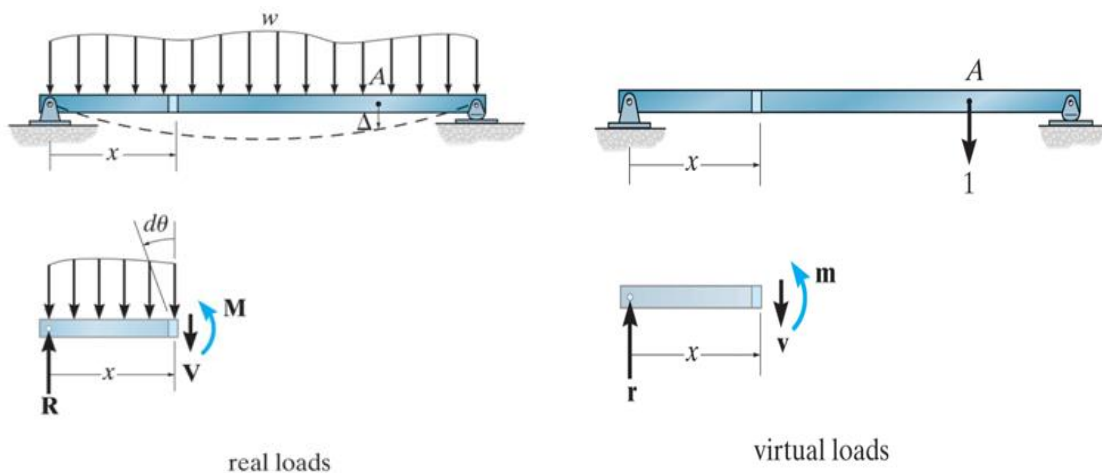
Real deflection is at "a" = Δ + Real internal rotation is $d\theta$.

Next, apply a virtual load ($P' = 1$) @ point where Δ is required \rightarrow internal virtual moment, m

$$\text{External work} = 1 * \Delta$$

$$\text{Internal virtual work} = m * d\theta$$

$$1 \cdot \Delta = \sum m \cdot d\theta$$



Deflection/Displacement:

Recalling that the moment is related to the slope by $d\theta = \frac{M}{EI} dx$,

$$1 \cdot \Delta = \int_0^L \frac{mM}{EI} dx$$

- 1 = external **virtual** unit load applied at point “a”
- m = internal moment caused by the external **virtual** force
- Δ_a = deflection at point “a’ due to **real** loads
- M = internal moment caused by the external **real** loads
- E, I = Modulus of elasticity of & moment of inertia of the beam cross-section

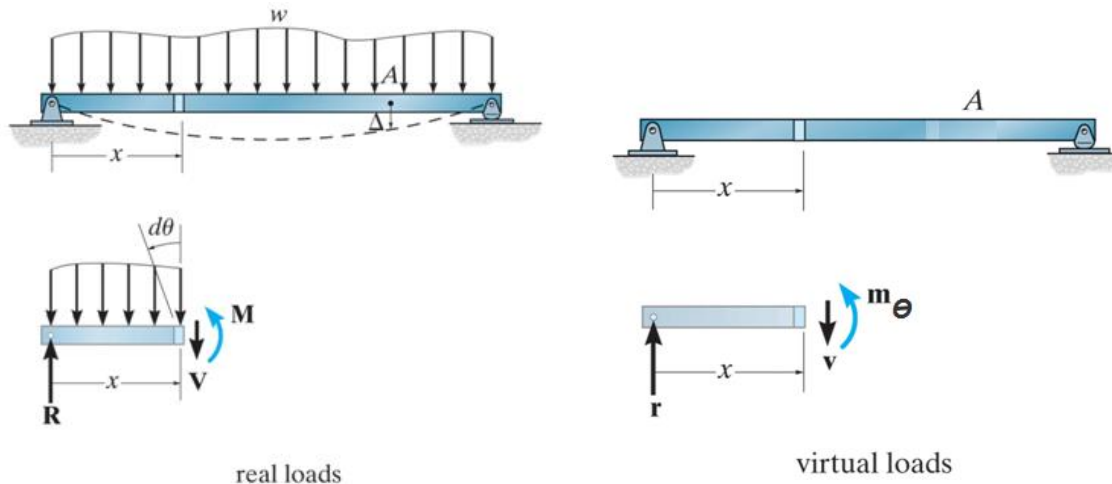
Slope:

In a similar manner, the **slope** at a point may be obtained using:

$$1 \cdot \theta = \int_0^L \frac{m_\theta M}{EI} dx$$

- 1 = external **virtual** unit couple applied at point “a”
- θ_a = slope at point “a’ due to **real** loads
- m_θ = internal moment caused by the external **virtual** couple
- M = internal moment caused by the external **real** loads
- E, I = Modulus of elasticity of & moment of inertia of the beam cross-section

Notice that the expression is the same except that we now apply a **virtual couple** at the point in question where the slope is desired.



2. Procedure for analysis

Procedure:

1. Ensure that beam/frame is determinate and stable:
 - Recall if : $3m + r = 3j + e_c \rightarrow$ beam/frame is determinate
2. Structure subjected to real forces:
 - With the **real** loads on the structure, use statics to determine internal moments M .
 - It may easier to treat different external loads separately and then add their effects using the principle of superposition
3. Structure subjected to virtual force:
 - Remove real loads
 - If **displacement** is desired:
 - Place a unit load (= 1 kN) at the point where the desired displacement is to be determined
 - If **slope** is desired:
 - Place a unit couple (= 1 kN m) at the point where the desired slope is to be determined
 - Calculate the internal moments m or m_θ due to the virtual load alone.
4. Apply virtual-work equation:

$$1 \cdot \Delta = \int_0^L \frac{mM}{EI} dx \quad \text{or} \quad 1 \cdot \theta = \int_0^L \frac{m_\theta M}{EI} dx$$

- Be careful to retain the algebraic sign for each of “m” and “M”
- Be careful to be consistent with units
 - $\text{mm}^2 = 1 \times 10^{-6} \text{ m}^2$
 - $\text{GPa} = 1 \times 10^6 \text{ kN/m}^2$
 - $\text{mm}^4 = 1 \times 10^{-12} \text{ m}^4$
- If the sum of the integrals is:
 - positive (+) : displacement/slope is in same direction as unit load/couple
 - negative (-) : displacement is in opposite direction as unit load/couple

Note: There are three options to evaluate $\int_b^a mM dx$

- **Option 1** : use **Direct Integration** ... i.e. actually solving the integrals mathematically
- **Option 2A:** Use **Graphical method** ... see p.4-5
- **Option 2B:** Use **Mohr's Integration tables** ... see p.6-7

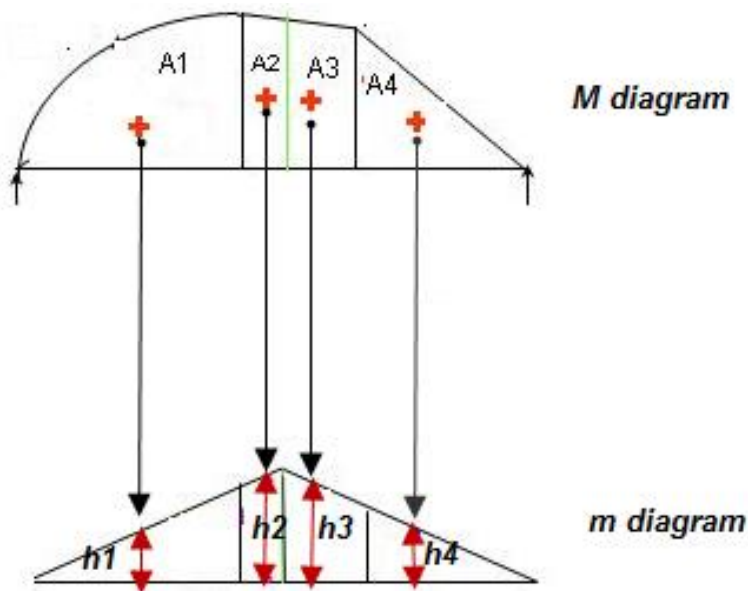
Option 2A: using **Graphical method to evaluate** $\int_b^a mMdx$

Rather than computing the integral using integration one can use a graphical method which is somewhat simpler and less time consuming.

Procedure:

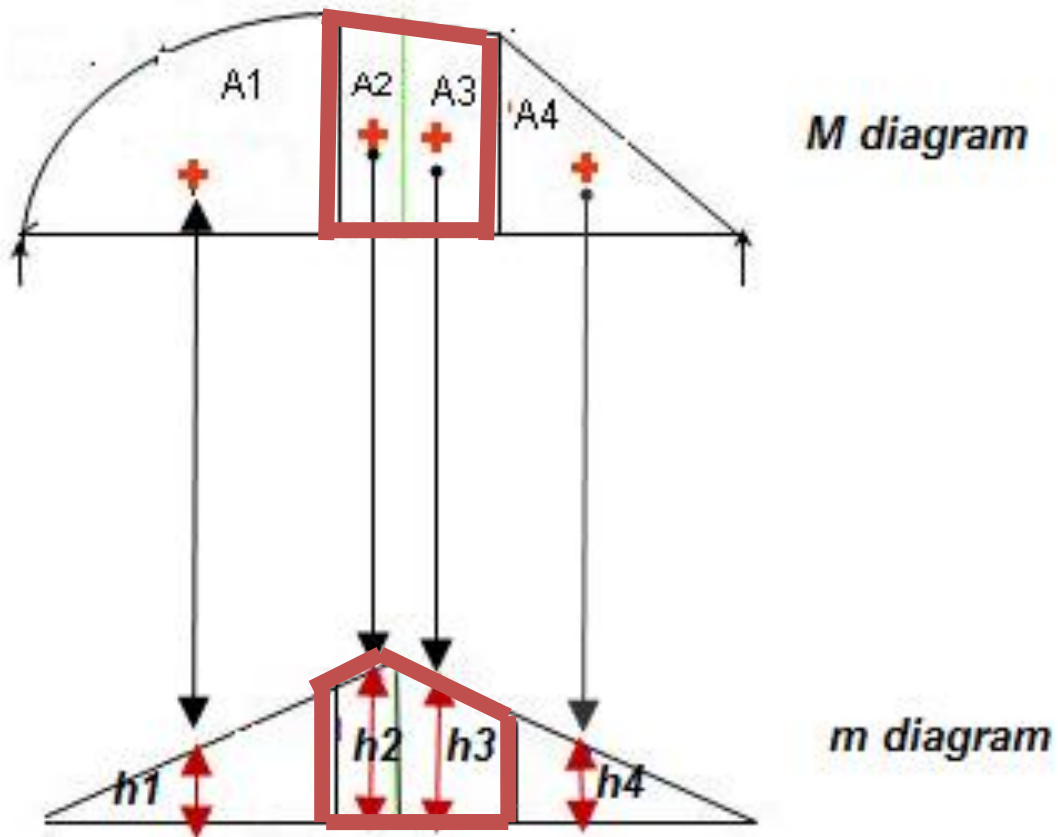
- 1) Draw the **M-diagram** for applied loading
 - a. if there is multiple loads, draw the M diagram by “parts”
- 2) Draw the **m-diagram** due to the unit virtual load/couple
- 3) Divide the **M-diagram**, into segments¹ for which you can easily find the area (e.g. triangle, rectangle, parabola ...)
- 4) For each segment:
 - a. Calculate the area A_i on the **M-diagram**
 - b. Locate the center of gravity CG_i on the **M-diagram**
 - c. Project the location of CG_i onto the **m-diagram**
 - d. Pick up the height, h_i , on the **m-diagram**
 - e. Compute $A_i * h_i$ for each segment

5) To get $\int_0^L \frac{mM}{EI} dx$, sum up the results for all the segments: $\frac{1}{EI} \sum_i A_i * h_i$



Note

- Both moment diagrams must be continuous over each segment length.
- If M or m diagram is not continuous, one MUST divide into more segments, each of which is continuous over the integration length



Option 2B: using Mohr's **integration tables to evaluate** $\int_b^a mMdx$

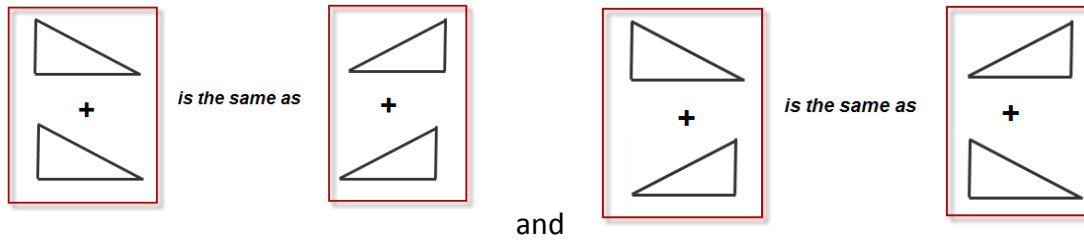
As an alternative we can use "Mohr" integration tables to evaluate the integral $\int_0^L mMdx$,

recognizing that it is of the format $\int_0^L f_1(x) * f_2(x)dx$.

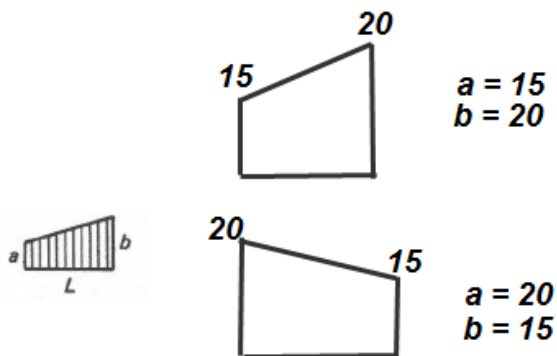
Procedure:

- 1) Complete steps 1-3 as before
- 2) Take the row value $f_1(x)$ that corresponds to either the "M" or "m" segment
- 3) Take the column value $f_2(x)$ that corresponds to either the "m" or "M" segment
- 4) Compute the area in the cell corresponding to the $f_1(x)$ & $f_2(x)$ combination
- 5) To get $\int_0^L \frac{mM}{EI} dx$, sum up the results for all the segments !

Note #1:



Note #2:



MOHR'S INTEGRATION TABLES

Note: Select a $f_1(x)$ and the matching $f_2(x)$... order doesn't matter ... i.e. $f_1(x)$ can be the row or column

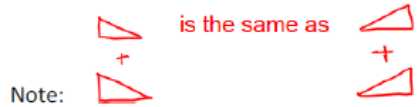


TABLE 7.1 VALUES OF $\int_0^L f_1(x)f_2(x)dx$

$f_2(x)$ $f_1(x)$				
	$\frac{1}{2} Lbc$	$\frac{1}{3} Lbd$	$\frac{Lb}{6} (c + 2d)$	$\frac{1}{3} Lbc$
	$\frac{1}{2} Lac$	$\frac{1}{6} Lad$	$\frac{La}{6} (2c + d)$	$\frac{1}{3} Lac$
	$\frac{L}{2} (a + b)c$	$\frac{Ld}{6} (a + 2b)$	$\frac{L}{6} (2ac + ad + 2bd + bc)$	$\frac{Lc}{3} (a + b)$
	$\frac{2}{3} Lac$	$\frac{1}{3} Lad$	$\frac{La}{3} (c + d)$	$\frac{8}{15} Lac$
	$\frac{2}{3} Lbc$	$\frac{5}{12} Lbd$	$\frac{Lb}{12} (3c + 5d)$	$\frac{7}{15} Lbc$
	$\frac{2}{3} Lac$	$\frac{1}{4} Lad$	$\frac{La}{12} (5c + 3d)$	$\frac{7}{15} Lac$
	$\frac{1}{3} Lbc$	$\frac{1}{4} Lbd$	$\frac{Lb}{12} (c + 3d)$	$\frac{1}{5} Lbc$
	$\frac{1}{3} Lac$	$\frac{1}{12} Lad$	$\frac{La}{12} (3c + d)$	$\frac{1}{5} Lac$
$\int f_2^2(x)dx$	Lc^2	$\frac{1}{3} Ld^2$	$\frac{L}{3} (c^2 + cd + d^2)$	$\frac{8}{15} Lc^2$

* Second-degree parabola.

3. Drawing M diagrams by parts:

Using superposition:

Superposition can be helpful if we plan to use Mohr's tables since if we have different loadings it can lead to complicated moment diagrams, and if the diagram is complicated we will not be able to find the combination of $M \cdot m$ on the Mohr's table sheet.

