



Final
EXAMINATION
December 2008

DURATION: 3 HOURS

No. of Students: 120

Department name & Course Number: Civil and Environmental Engineering Department

Mechanics of Solids – CIVE 2200

Course Instructor: Dr. Ehab Zalok



AUTHORIZED MEMORANDA

- (1) Closed book exam.
- (2) Formula sheet is provided, and
 - ✓ Hand in the formula sheet with your exam paper.
- (3) Programmable calculators are NOT allowed.
- (4) Questions carry equal weight.
- (5) Answer all questions.

Students **MUST** count the number of pages in this examination question before beginning to write, and report any discrepancy to a proctor. This question paper has **5** pages.

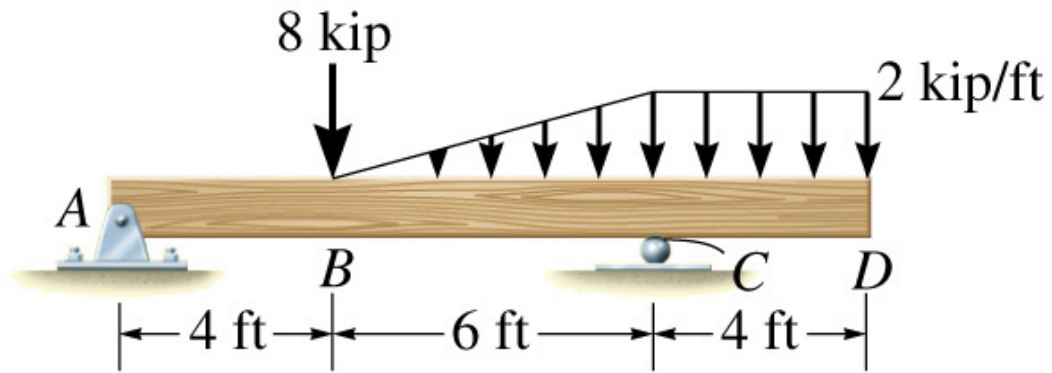
This examination question paper **MAY NOT** be taken from the examination room.

In addition to this question paper, students require: an additional booklet
a Scantron sheet

yes no
yes no

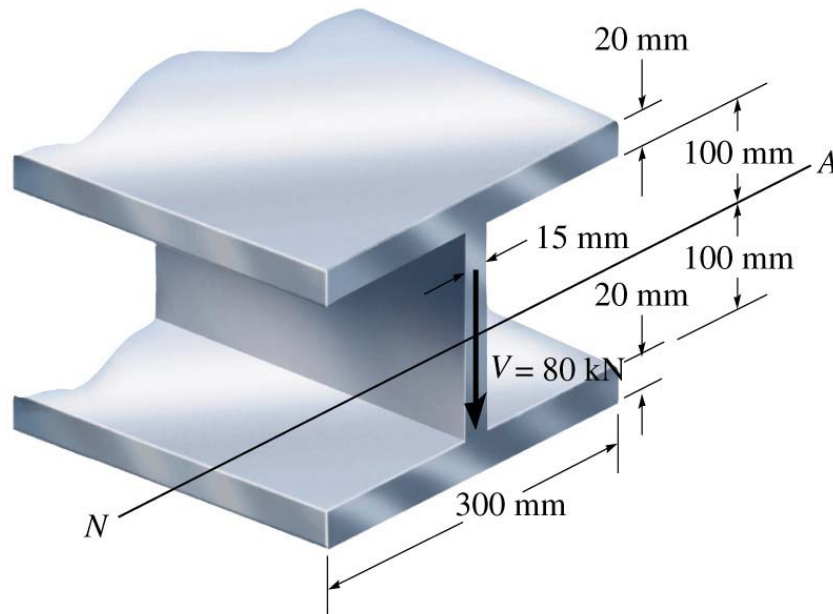
Question One

Draw the shear and moment diagrams for the overhanging beam shown in the figure below.



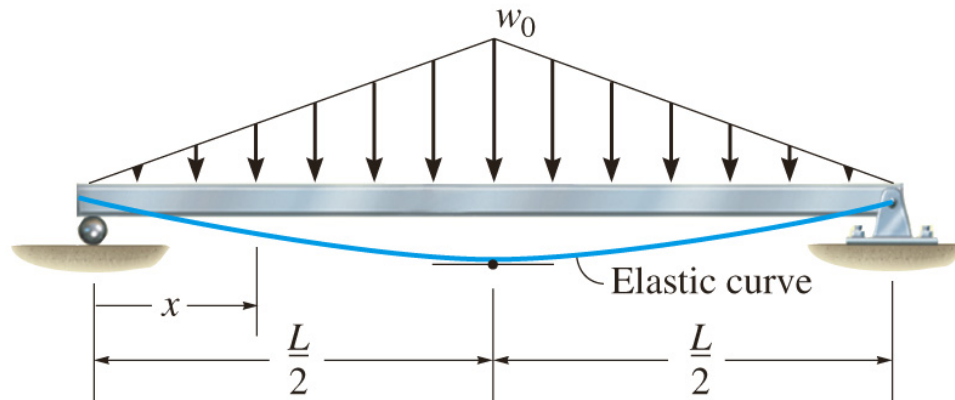
Question two

A steel wide-flange beam has the dimensions shown in the figure below. If it is subjected to a shear of $V = 80$ kN; plot the shear-stress distribution acting over the beam's cross-sectional area.



Question Three

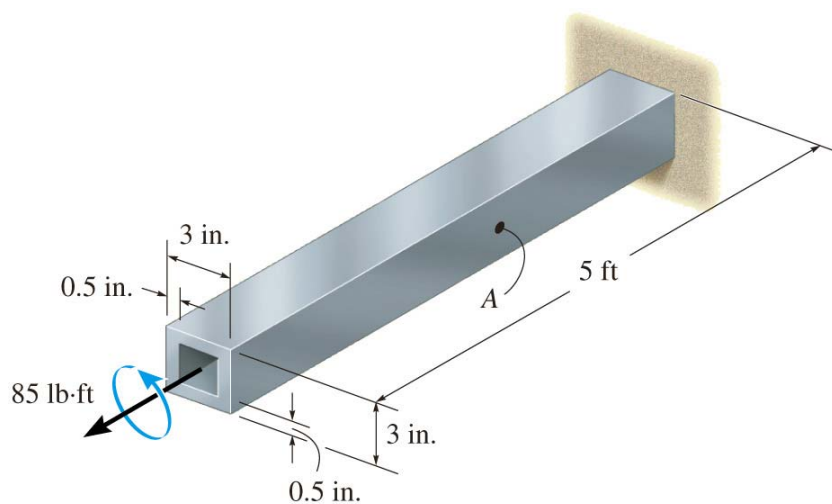
The simply supported beam shown below supports the triangular distributed loading. Determine its maximum deflection. EI is constant.



Question Four

A square aluminum tube has the dimensions shown in Figure below.

- Determine the average shear stress in the tube at point A if it is subjected to a torque of 85 lb-ft.
- Compute the angle of twist due to this loading.
Take $G_{al}=3.80 (10^3)$ ksi.

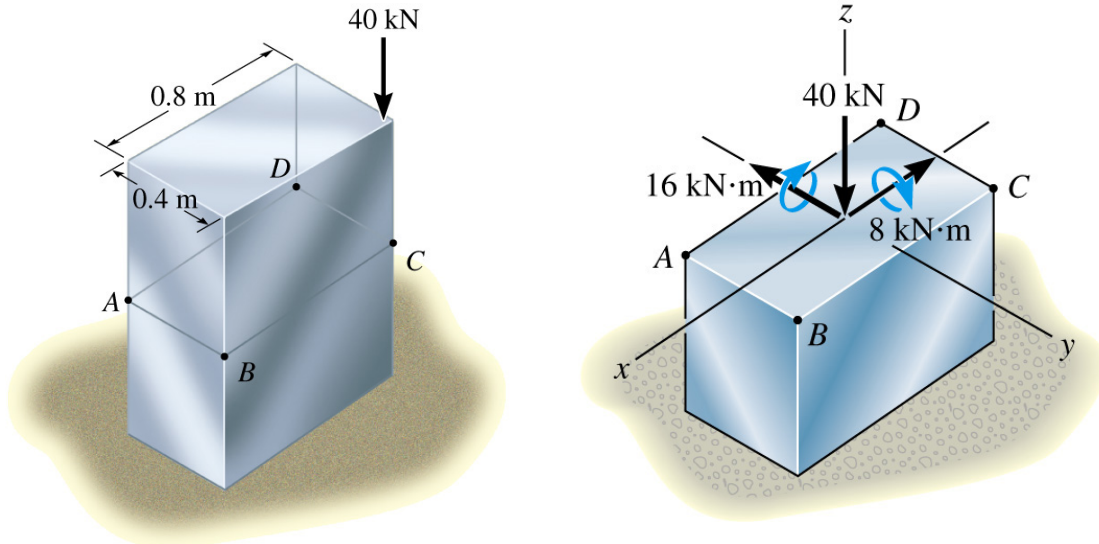


Question Five

The rectangular block of negligible weight in the left Figure is subjected to a vertical force of 40 kN, which is applied to its corner. The right Figure shows the effect of the 40 kN force at a section through ABCD.

Determine the normal-stress distribution acting on the section through ABCD.

HINT: Calculate the stresses at points A, B, C, and D.

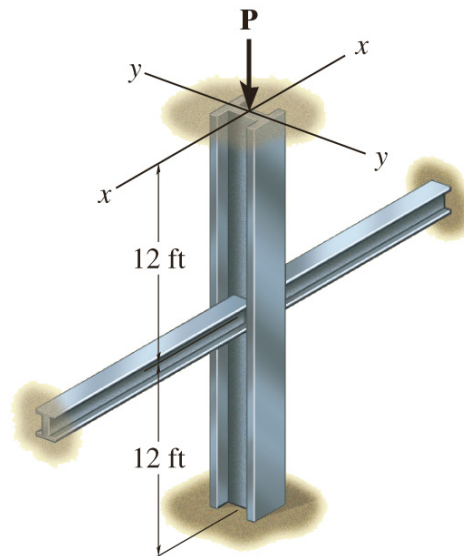


Question Six

A W6 x 15 steel column is 24 ft long and is fixed at its ends as shown in the figure. Its load-carrying is increased by bracing it about the y-y axis using struts that are assumed to be pin-connected to its midheight.

Determine the load it can support so that (a) the column does not buckle nor (b) the material exceed the yield stress.

For the W6 x 15 section; take: $I_x = 29.1 \text{ in}^4$, $I_y = 9.32 \text{ in}^4$, $E_{st} = 29(10^3) \text{ ksi}$, $\sigma_y = 60 \text{ ksi}$, $A = 4.43 \text{ in}^2$



Question Seven

About Mohr's Circle:

Axial Loading

Normal stress

$$\sigma = \frac{P}{A}$$

Displacement

$$\Delta = \frac{PL}{AE}$$

Torsion

Shear stress in a circular shaft

$$\tau = \frac{T\rho}{I_p}$$

Angle of twist of circular member

$$\phi = \frac{TL}{I_p G}$$

Shear stress in a rectangular shaft

$$\tau = \frac{T}{\alpha b r^2}$$

Shear stress in a thin-walled tube

$$\tau = \frac{T}{2t(\Delta)}$$

Stress Transformation

Principal normal stresses

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

Maximum and minimum shear stresses

$$\tau_{\max/\min} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

Bending

Normal stress in a straight member

$$\sigma = -\frac{My}{I}$$

Unsymmetric bending

$$\sigma_x = -\frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\tan \beta = \frac{I_z}{I_y} \tan \alpha$$

Shear

Shear flow and stress due to shear force

$$q = \tau t = \frac{VQ}{I}, \quad \tau = \frac{VQ}{It}$$

Mechanical Properties of Material

Poisson's ratio

$$\nu = -\frac{\epsilon_{\text{lat}}}{\epsilon_{\text{long}}}$$

Generalized Hooke's Law

$$\epsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)], \quad \gamma_{xy} = \frac{\tau_{xy}}{G}$$

$$\epsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)], \quad \gamma_{yz} = \frac{\tau_{yz}}{G}$$

$$\epsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)], \quad \gamma_{zx} = \frac{\tau_{zx}}{G}$$

where shear modulus

$$G = \frac{E}{2(1 + \nu)}$$

Dilatation

$$e = \frac{1 - 2\nu}{E} (\sigma_x + \sigma_y + \sigma_z)$$

Bulk modulus

$$k = \frac{E}{3(1 - 2\nu)}$$

Deflection of Beams

Relations between q , V , M

$$\frac{dV}{dx} = q(x), \quad \frac{dM}{dx} = V(x)$$

Elastic curve

$$\frac{1}{\rho} = \frac{M}{EI}, \quad EI \frac{d^4 v}{dx^4} = q(x)$$

$$EI \frac{d^2 v}{dx^2} = V(x), \quad EI \frac{d^2 v}{dx^2} = M(x)$$

Buckling

Critical axial load

$$P_{cr} = \frac{\pi^2 EI}{(KL)^2}$$

Critical axial stress

$$\sigma_{cr} = \frac{P_{cr}}{A} = \frac{\pi^2 E}{(KL/r)^2}$$

Radius of gyration

$$r = r_{\min} = \sqrt{\frac{I_{\min}}{A}}$$