

# CIVL 311

## U-U laboratory testing of soils

### Laboratory shear testing of soils

#### ■ Step 1: Specimen preparation

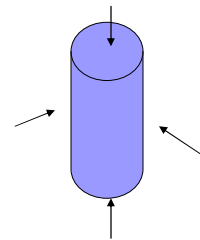
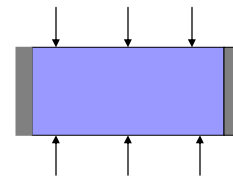
- Undisturbed specimen (usually for cohesive soils)
- Reconstituted specimen (usually for cohesionless soils )



## Laboratory shear testing of soils

### ■ Step 2: Consolidation

- 1D consolidation
- Hydrostatic consolidation

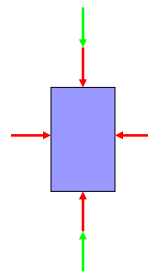
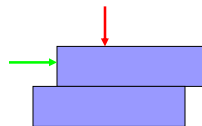


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## Laboratory shear testing of soils

### ■ Step 3: Shearing

- By applying direct shear
- By increasing deviator stress



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## Undrained shear testing of soils

- Undrained shear testing correspond to situations when load is applied rapidly and water does not have enough time to escape.
- Examples:
  - End of construction on low permeability soils
  - Earthquakes loading
  - Rapid lowering of water level behind a dam

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## Laboratory shear tests

- Unconfined compression test
- Direct shear test
- Triaxial test
- Simple shear test
- Hollow cylindrical test
- Ring shear test

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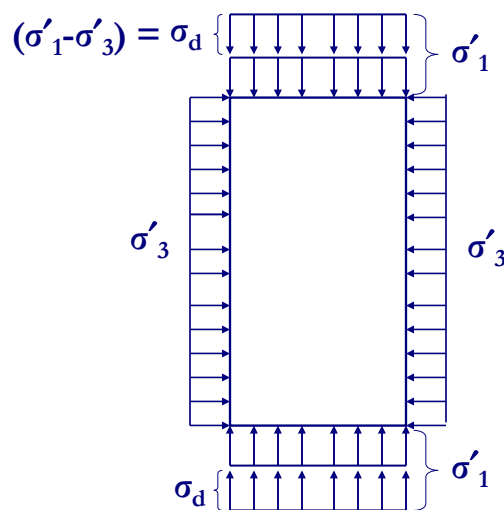
# Unconfined compression test

- Special case of triaxial test
- No lateral support
- Advantages:
  - Simple
- Limitations:
  - Effective stress state is not known
  - Only for cohesive soils



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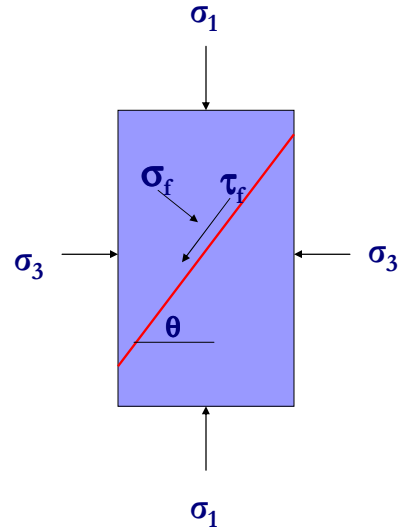
# Triaxial testing



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## Triaxial testing

- No predefined failure plane
- We only know the magnitude and direction of principal stresses
- Hence, we need to find a relation between principal stresses and the stresses acting on the plane of failure to be able to draw the failure envelop
- We know  $\sigma_1$   $\sigma_3$
- We want to get  $\sigma_f$   $\tau_f$



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## Triaxial testing: 2 Phases

- **CONSOLIDATION**
  - Apply equal, all-around pressure
  - Drained or undrained
- **SHEARING**
  - Apply axial stress (until failure)
  - Drained or undrained

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# Triaxial Test Types

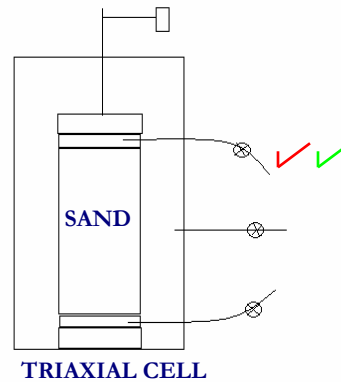
Consolidation  
phase

Shearing  
phase

■ C

D

Sands and clays long  
time after  
construction



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# Triaxial Test Types

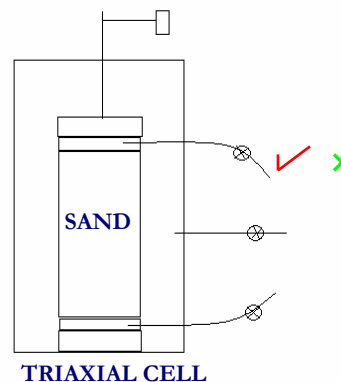
Consolidation  
phase

Shearing  
phase

■ C

D

Sands and clays long  
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# Triaxial Test Types

Consolidation  
phase

Shearing  
phase

■ C

D

Sands and clays long  
time after  
construction

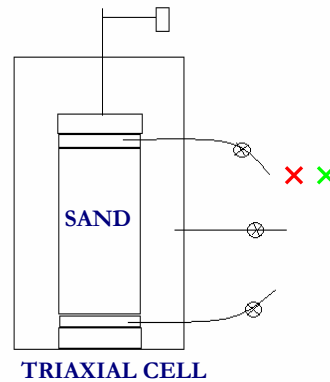
■ C

U

Clays short time after  
construction

■ U

U



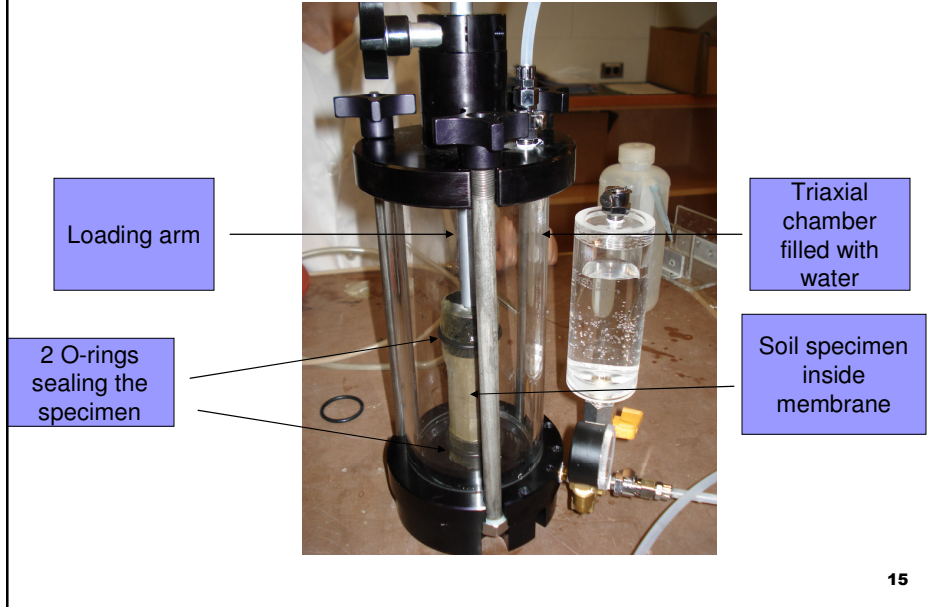
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## U-U triaxial testing (specimen preparation )

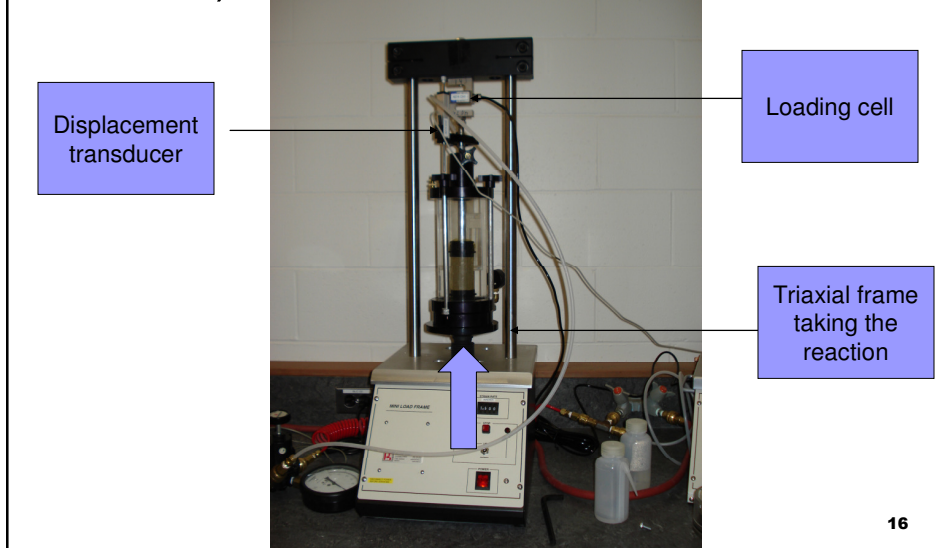


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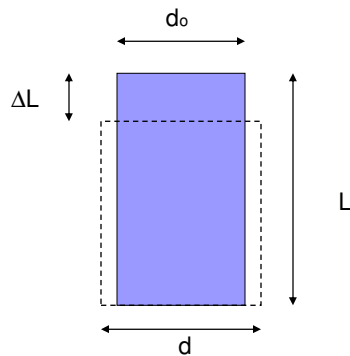
## U-U Triaxial testing (Triaxial cell)



## U-U Triaxial testing (Triaxial loading frame )



## Calculations



$$\varepsilon = \frac{\Delta L}{L}$$

$$A = \frac{A_o}{1 - \varepsilon} = \frac{\pi}{4} d^2$$

$$\sigma_1 = \frac{F_a}{A}$$

$\sigma_3$  = Cell Pressure

Deviator stress ( $\sigma_d$ ) =  $\sigma_1 - \sigma_3$

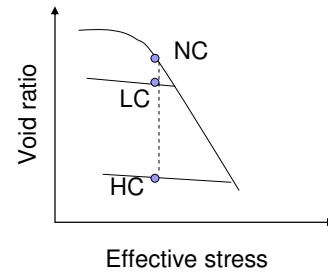
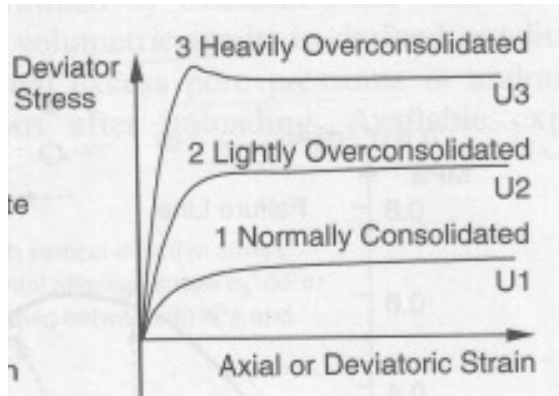
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## Calculations

- Water content before and after the test. This should tell if there is any leakage.
- Degree of saturation. For undrained conditions the degree of saturation must be 100%

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## Deviator stress vs. axial strain

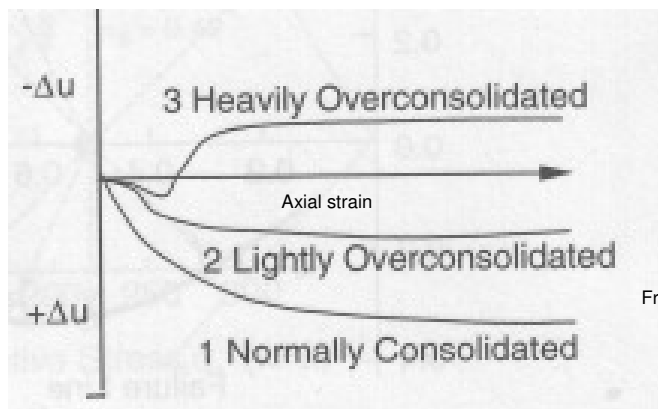


From Mitchell and Soga  
(2005)

Stiffer response for OCC and higher strength

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## Excess PWP due to undrained shearing

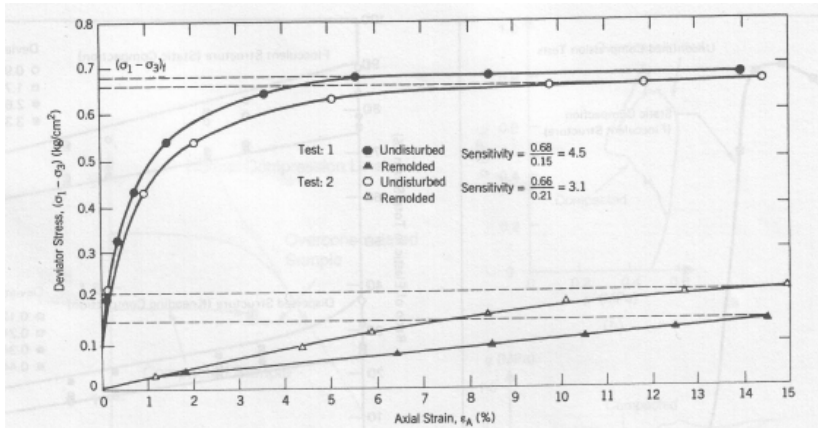


From Mitchell and Soga  
(2005)

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# Clay sensitivity

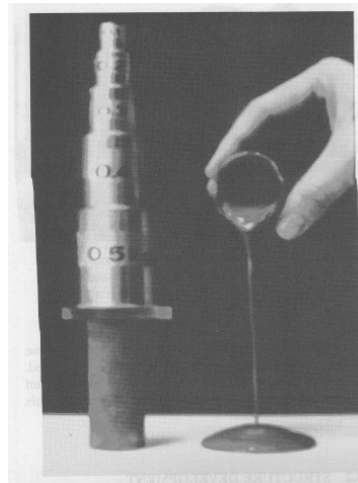
From Mitchell and Soga (2005)



Clay sensitivity = Strength undisturbed/ strength remoulded

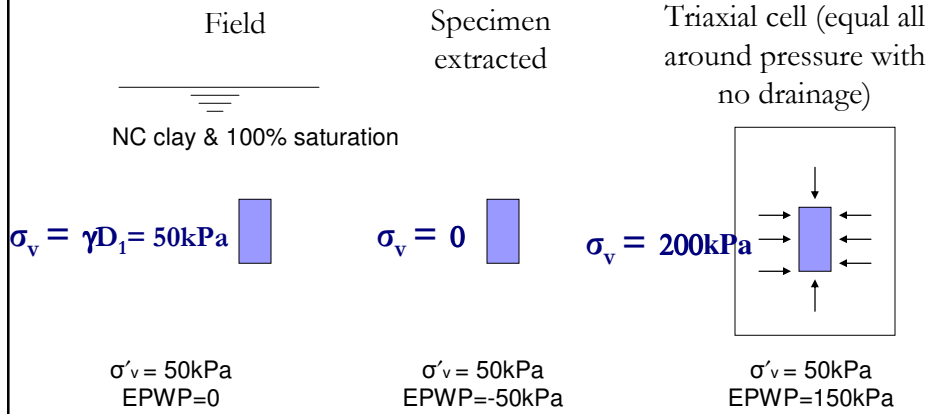
# Clay sensitivity

- S = 1      Insensitive
- S=1-2     Slightly sensitive
- S=2-4     Medium sensitive
- S=4-8     Very sensitive
- S>8       Quick clay



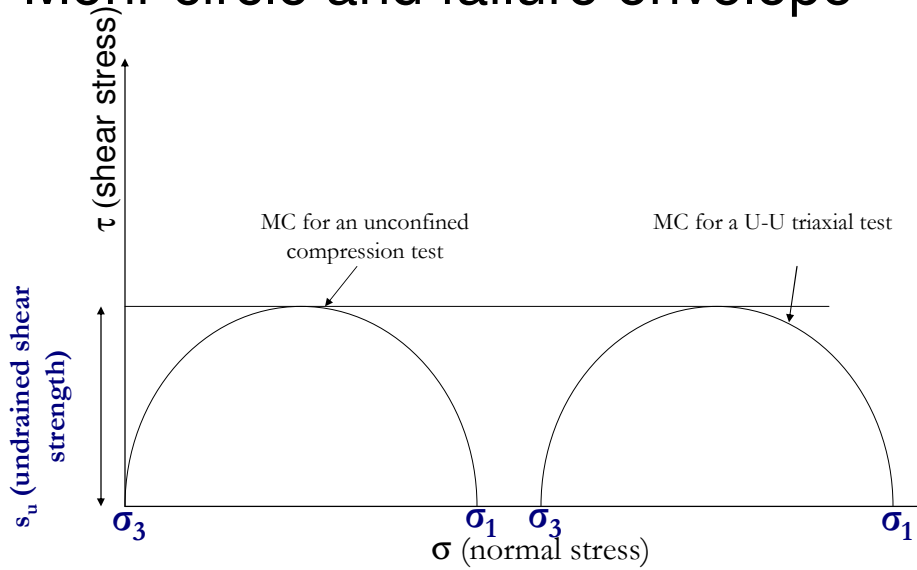
From DAS (2002)

# Example



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# Mohr circle and failure envelope



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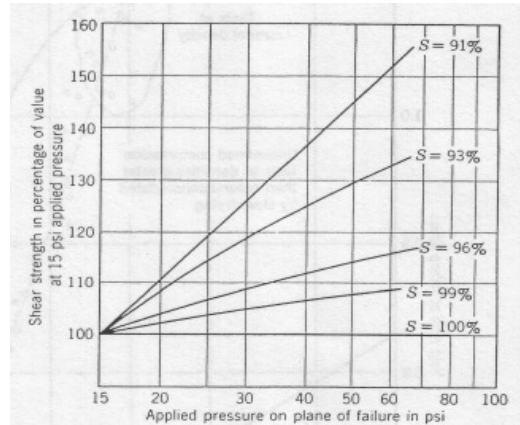
## Typical $s_u$ values

0-12.5 kPa	very soft clay
12.5-25 kPa	soft clay
25-50 kPa	medium clay
50-100 kPa	stiff clay
100-200 kPa	very stiff clay
>200	hard clay

From DAS (2002)

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## Effect of the degree of saturation



From Soil Testing for Engineers by Lambe

Fully undrained behaviour is at 100% saturation. Shear strength is insensitive to applied pressure.

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## Reporting

### ■ Introduction

- Name of the experiment
- Soil description and properties
- Significance of the test (i.e. U-U test can be used to obtain  $s_u$  and stress-strain curve)
- Applicable to rapid loading conditions

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## Reporting (con't)

### ■ Method statement

- Brief summary of U-U testing procedure in your own words
- Reference

### ■ Results

- Tabulated results: WC (initial and final), degree of saturation,  $s_u$  for U-U tests at different confining pressures, and sensitivity.

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## Reporting (con't)

- Graphs:
  - Deviator stress vs. axial strain for 2 U-U tests
  - Mohr circles and total stress failure envelope
- Discussion
  - Brief description of the difference between the different types of triaxial tests.
  - Expected behaviour according to the book:
    - Total stress envelope
    - Deviator stress vs strain

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## Reporting (con't)

- Observed behaviour and how does it compare with expected behaviour ? If not matching, what are the possible reasons?
- Typical  $s_u$  values
- Comment on clay sensitivity
- Comment on degree of saturation
- Effect of loading rate
- Any difference between WC of the two tests?
- Any difference between WC initial and WC final?

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## Reporting (con't)

- Any problems during the experiment? If yes, how are they expected to affect the results?
- Advantages and limitations of the U-U triaxial test.
- Calculations
  - Axial strain
  - Corrected area
  - Axial and deviator stress
  - WC, degree of saturation and clay sensitivity

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