

### CHAPTER 2

### Stress and Strain-Axial Loading



## Stress & Strain: Axial Loading

- Suitability of a structure or machine may depend on the deformations in the structure as well as the stresses induced under loading. Statics analyses alone are not sufficient.
- Considering structures as deformable allows determination of member forces and reactions which are statically indeterminate.
- Determination of the stress distribution within a member also requires consideration of deformations in the member.
- Chapter 2 is concerned with deformation of a structural member under axial loading. Later chapters will deal with torsional and pure bending loads.

## Normal Strain

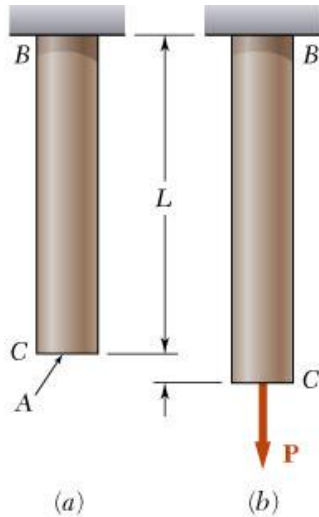


Fig. 2.1

$$\sigma = \frac{P}{A} = \text{stress}$$

$$\varepsilon = \frac{\delta}{L} = \text{normal strain}$$

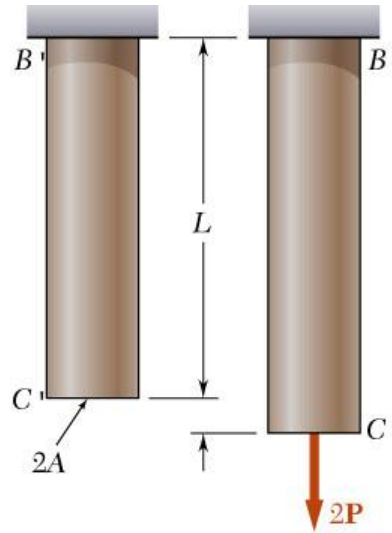


Fig. 2.3

$$\sigma = \frac{2P}{2A} = \frac{P}{A}$$

$$\varepsilon = \frac{\delta}{L}$$

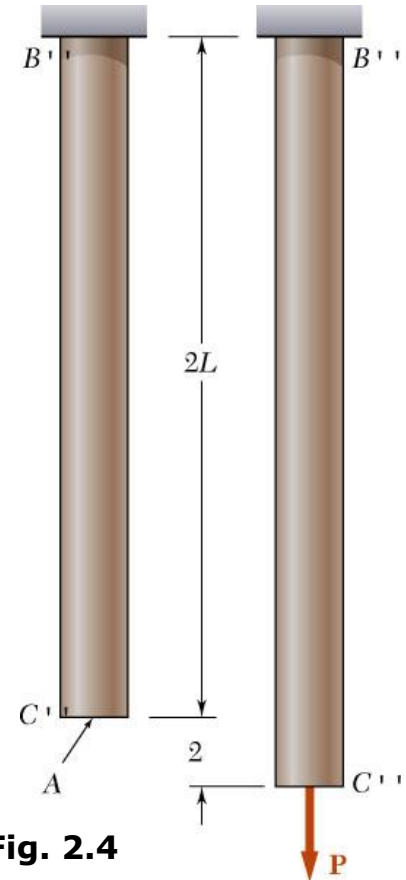
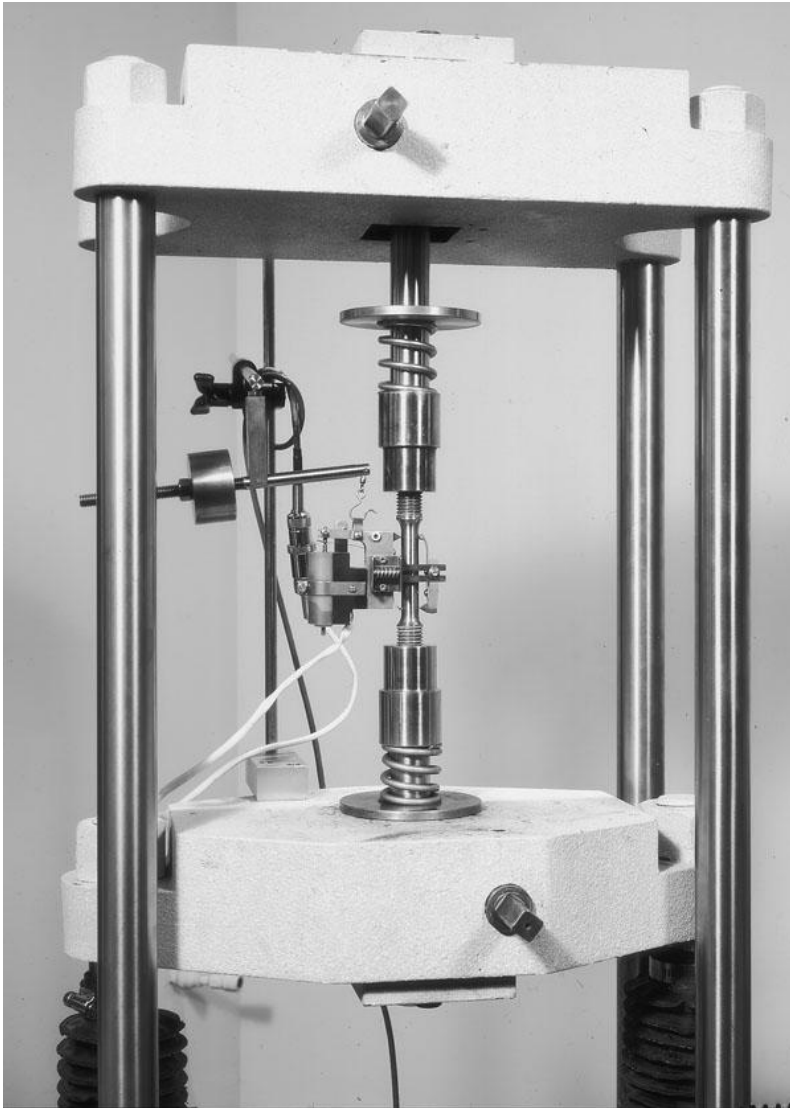


Fig. 2.4

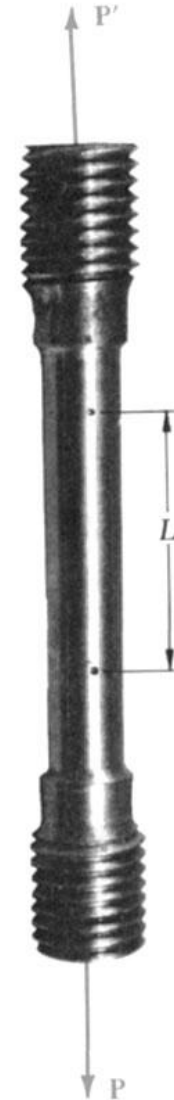
$$\sigma = \frac{P}{A}$$

$$\varepsilon = \frac{2\delta}{2L} = \frac{\delta}{L}$$

## Stress-Strain Test

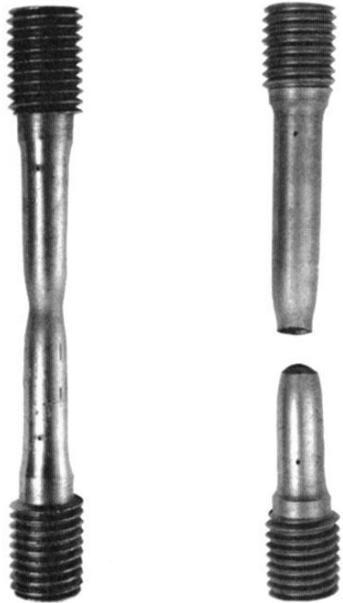


**Fig 2.7** This machine is used to test tensile test specimens, such as those shown in this chapter.



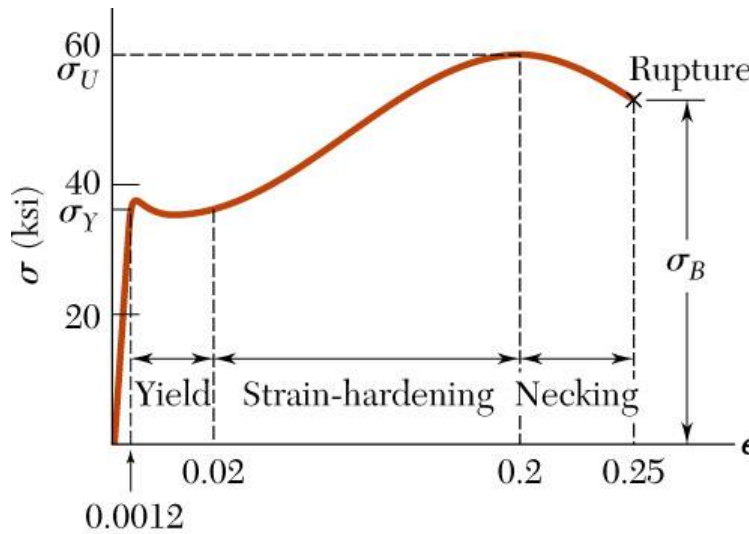
**Fig 2.8** Test specimen with tensile load.

## Stress-Strain Diagram: Ductile Materials

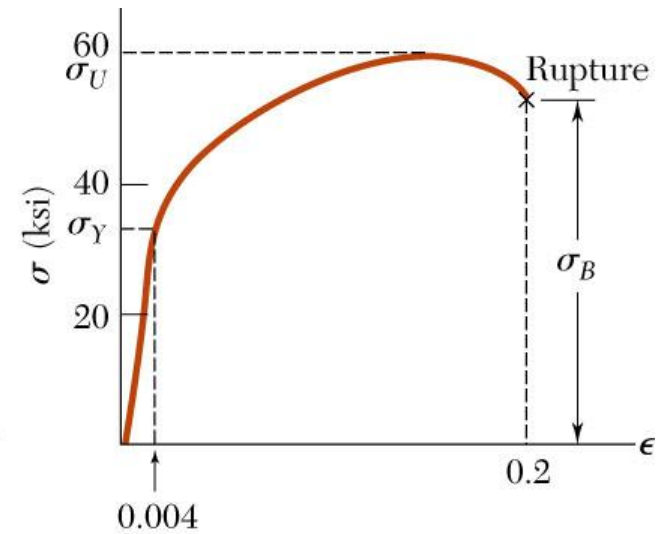


(a)

(b)

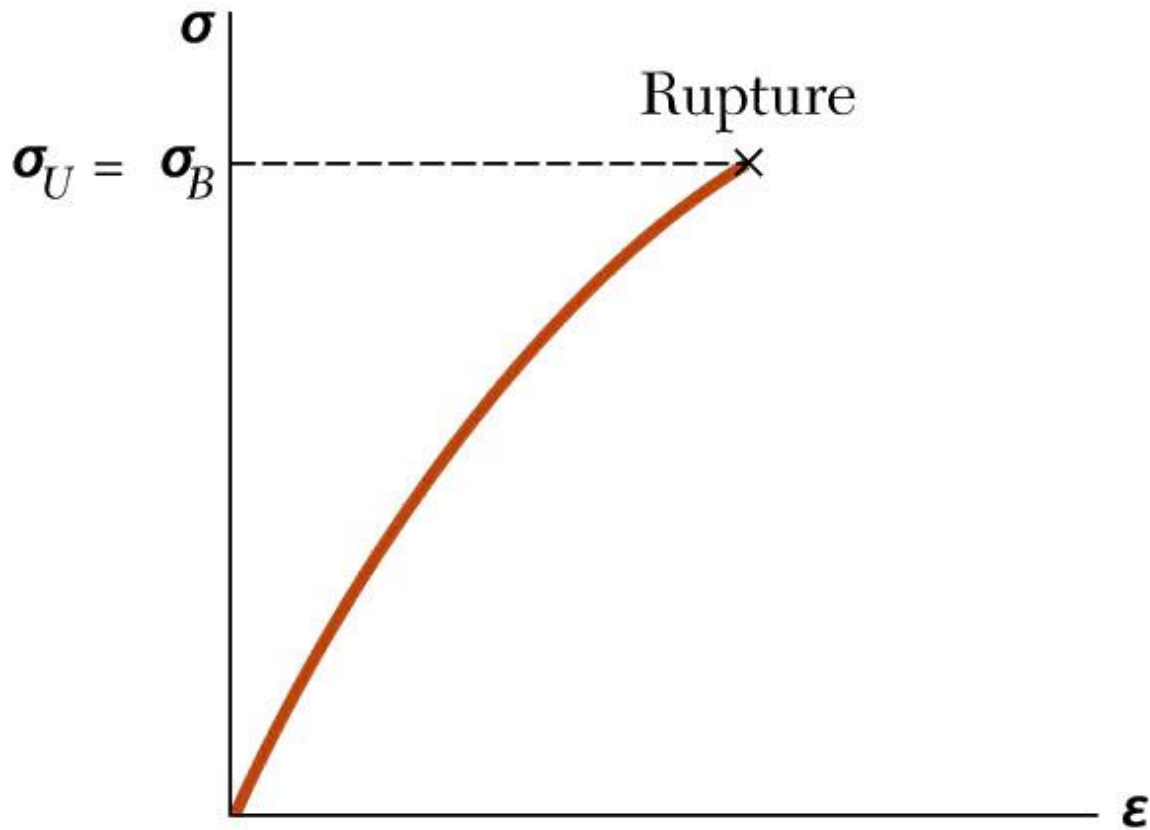
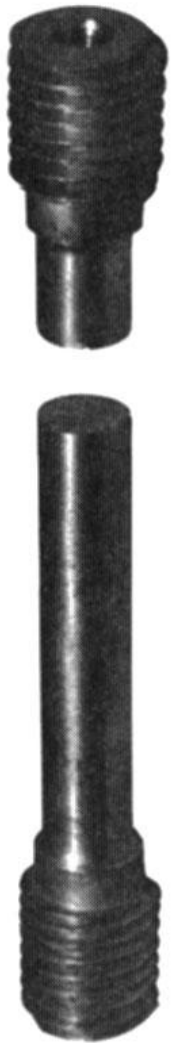


(a) Low-carbon steel



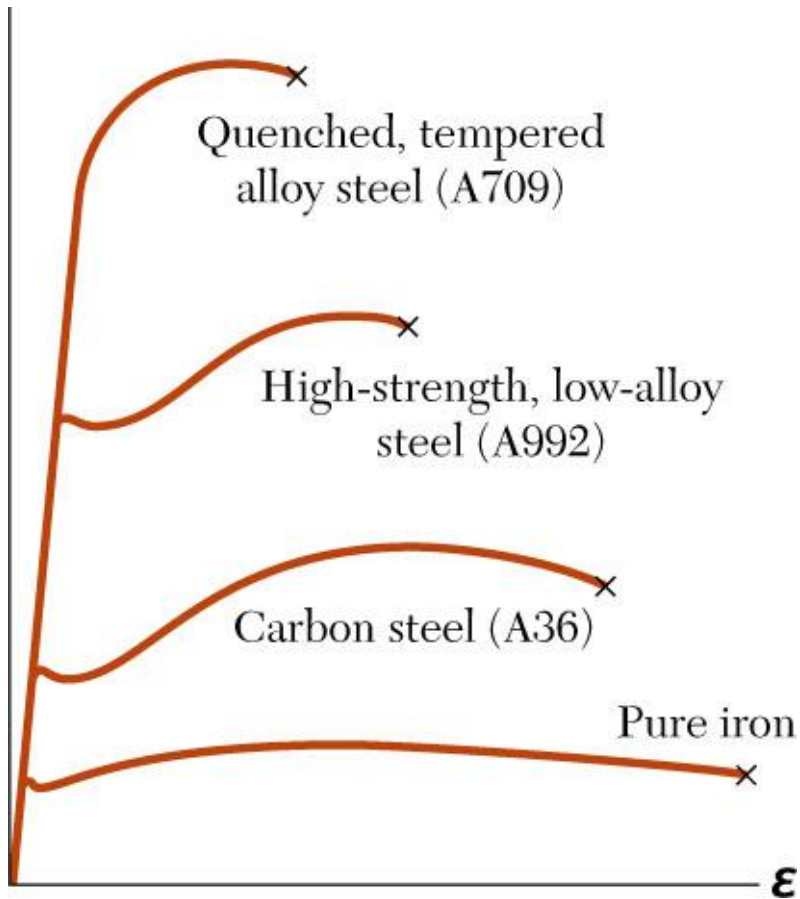
(b) Aluminum alloy

## Stress-Strain Diagram: Brittle Materials



**Fig 2.1** Stress-strain diagram for a typical brittle material.

## Hooke's Law: Modulus of Elasticity



**Fig 2.16** Stress-strain diagrams for iron and different grades of steel.

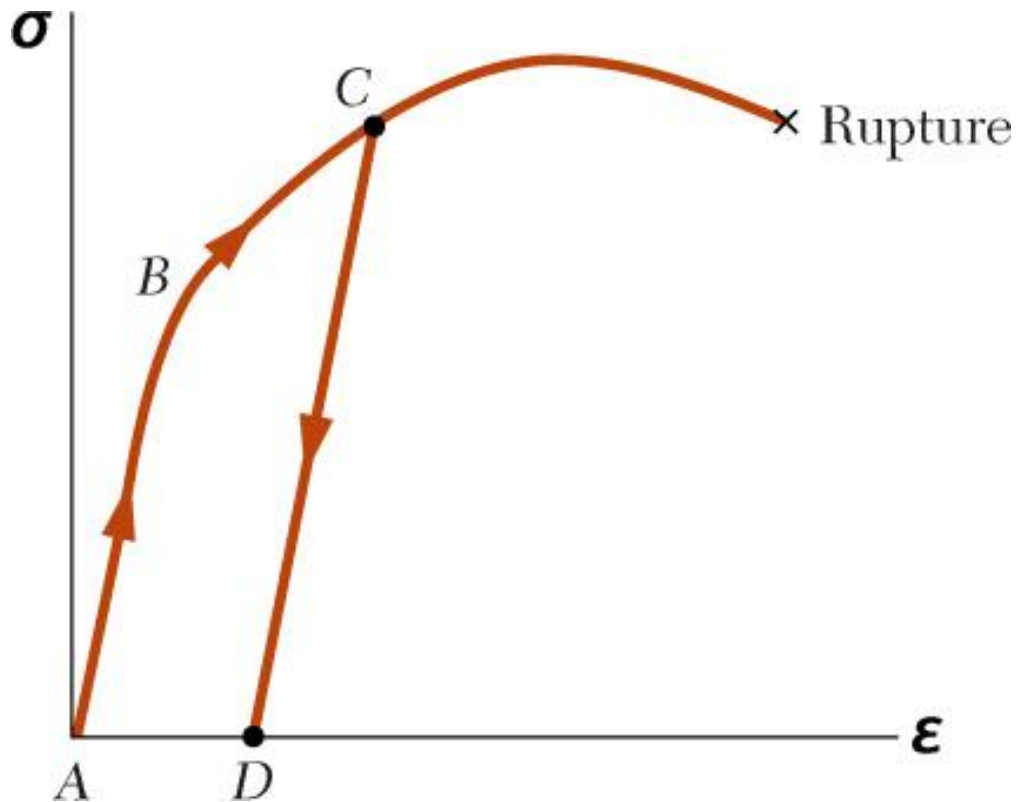
- Below the yield stress

$$\sigma = E\epsilon$$

$E$  = Youngs Modulus or  
Modulus of Elasticity

- Strength is affected by alloying, heat treating, and manufacturing process but stiffness (Modulus of Elasticity) is not.

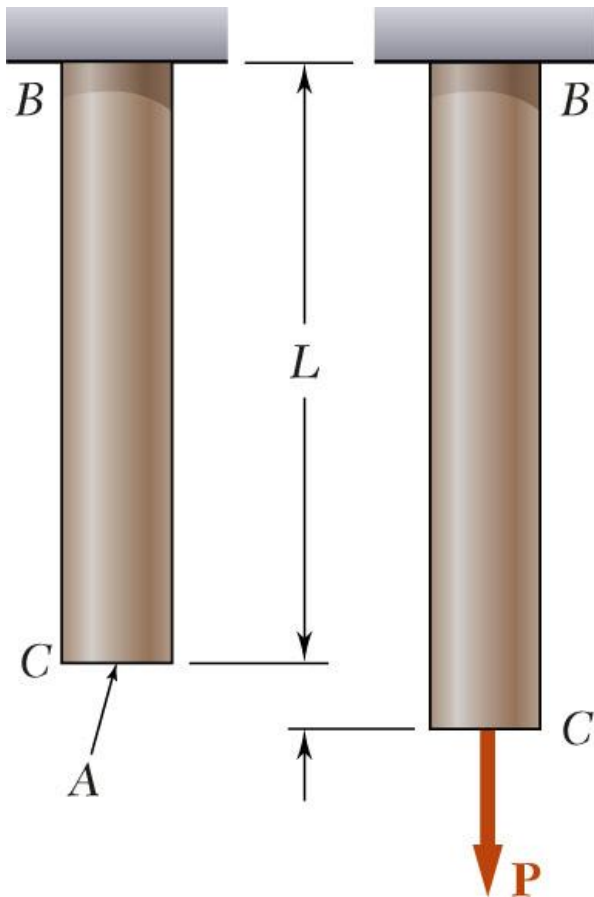
## Elastic vs. Plastic Behavior



- If the strain disappears when the stress is removed, the material is said to behave *elastically*.
- The largest stress for which this occurs is called the *elastic limit*.
- When the strain does not return to zero after the stress is removed, the material is said to behave *plastically*.

**Fig. 2.18**

## Deformations Under Axial Loading



**Fig. 2.22**

- From Hooke's Law:

$$\sigma = E\varepsilon \quad \varepsilon = \frac{\sigma}{E} = \frac{P}{AE}$$

- From the definition of strain:

$$\varepsilon = \frac{\delta}{L}$$

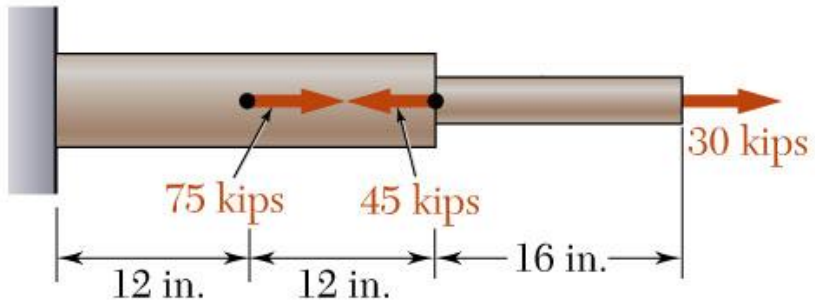
- Equating and solving for the deformation,

$$\delta = \frac{PL}{AE}$$

- With variations in loading, cross-section or material properties,

$$\delta = \sum_i \frac{P_i L_i}{A_i E_i}$$

## Example 2.01



$$E = 29 \times 10^6 \text{ psi}$$

$$D = 1.07 \text{ in.} \quad d = 0.618 \text{ in.}$$

Determine the deformation of the steel rod shown under the given loads.

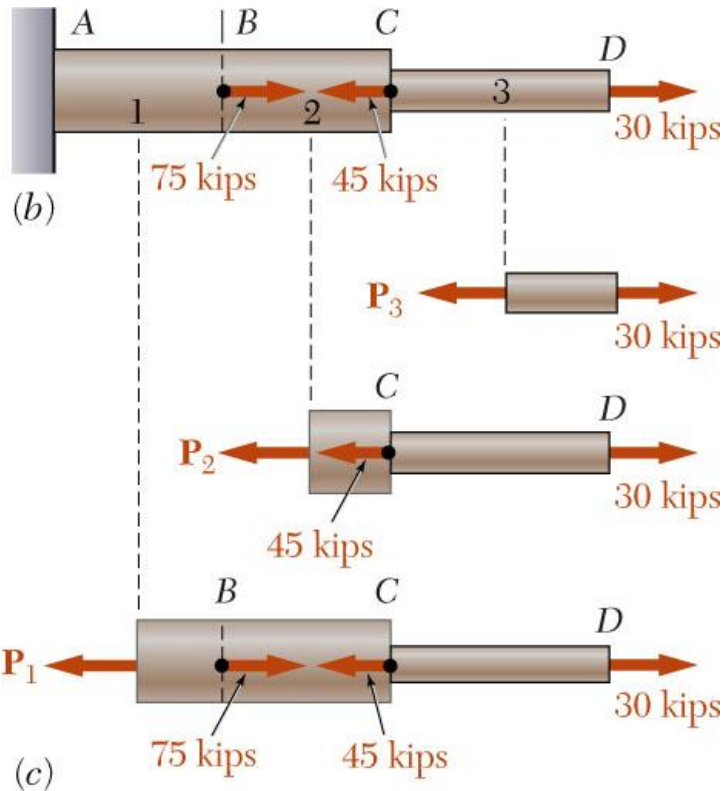
### SOLUTION:

- Divide the rod into components at the load application points.
- Apply a free-body analysis on each component to determine the internal force
- Evaluate the total of the component deflections.

# MECHANICS OF MATERIALS

## SOLUTION:

- Divide the rod into three components:



$$L_1 = L_2 = 12 \text{ in.} \quad L_3 = 16 \text{ in.}$$

$$A_1 = A_2 = 0.9 \text{ in}^2 \quad A_3 = 0.3 \text{ in}^2$$

- Apply free-body analysis to each component to determine internal forces,

$$P_1 = 60 \times 10^3 \text{ lb}$$

$$P_2 = -15 \times 10^3 \text{ lb}$$

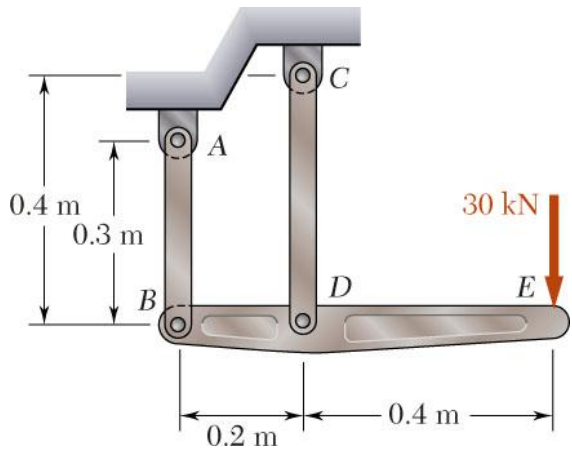
$$P_3 = 30 \times 10^3 \text{ lb}$$

- Evaluate total deflection,

$$\begin{aligned} \delta &= \sum_i \frac{P_i L_i}{A_i E_i} = \frac{1}{E} \left( \frac{P_1 L_1}{A_1} + \frac{P_2 L_2}{A_2} + \frac{P_3 L_3}{A_3} \right) \\ &= \frac{1}{29 \times 10^6} \left[ \frac{(60 \times 10^3) 12}{0.9} + \frac{(-15 \times 10^3) 12}{0.9} + \frac{(30 \times 10^3) 16}{0.3} \right] \\ &= 75.9 \times 10^{-3} \text{ in.} \end{aligned}$$

$$\delta = 75.9 \times 10^{-3} \text{ in.}$$

## Sample Problem 2.1



The rigid bar  $BDE$  is supported by two links  $AB$  and  $CD$ .

Link  $AB$  is made of aluminum ( $E = 70$  GPa) and has a cross-sectional area of  $500 \text{ mm}^2$ . Link  $CD$  is made of steel ( $E = 200$  GPa) and has a cross-sectional area of ( $600 \text{ mm}^2$ ).

For the 30-kN force shown, determine the deflection a) of  $B$ , b) of  $D$ , and c) of  $E$ .

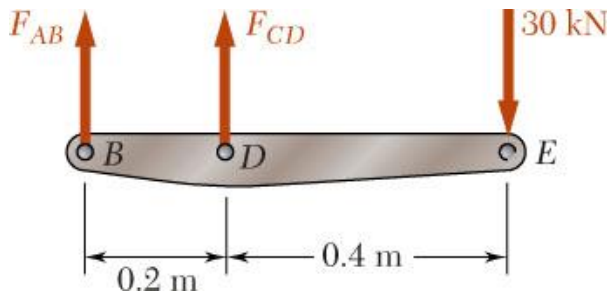
SOLUTION:

- Apply a free-body analysis to the bar  $BDE$  to find the forces exerted by links  $AB$  and  $CD$ .
- Evaluate the deformation of links  $AB$  and  $CD$  or the displacements of  $B$  and  $D$ .
- Work out the geometry to find the deflection at  $E$  given the deflections at  $B$  and  $D$ .

## Sample Problem 2.1

SOLUTION:

Free body: Bar *BDE*



$$+\curvearrowright \sum M_B = 0$$

$$0 = -(30 \text{ kN} \times 0.6 \text{ m}) + F_{CD} \times 0.2 \text{ m}$$

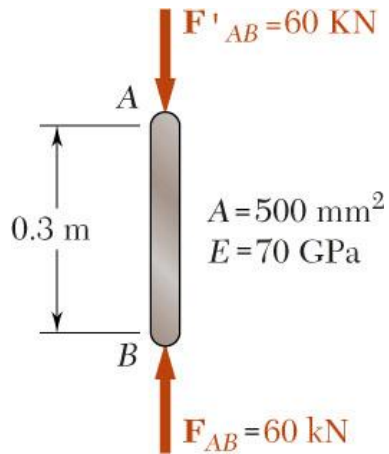
$$F_{CD} = +90 \text{ kN} \text{ tension}$$

$$+\curvearrowright \sum M_D = 0$$

$$0 = -(30 \text{ kN} \times 0.4 \text{ m}) - F_{AB} \times 0.2 \text{ m}$$

$$F_{AB} = -60 \text{ kN} \text{ compression}$$

Displacement of *B*:



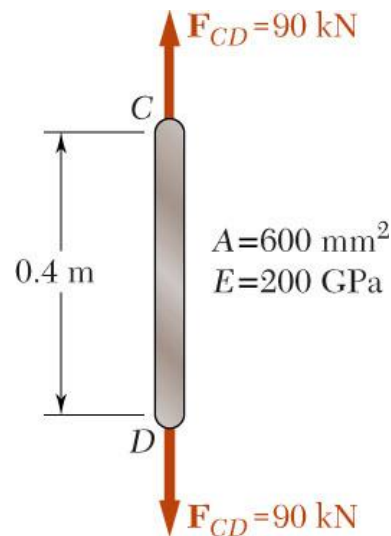
$$\delta_B = \frac{PL}{AE}$$

$$= \frac{(-60 \times 10^3 \text{ N})(0.3 \text{ m})}{(500 \times 10^{-6} \text{ m}^2)(70 \times 10^9 \text{ Pa})}$$

$$= -514 \times 10^{-6} \text{ m}$$

$$\delta_B = 0.514 \text{ mm} \uparrow$$

Displacement of *D*:



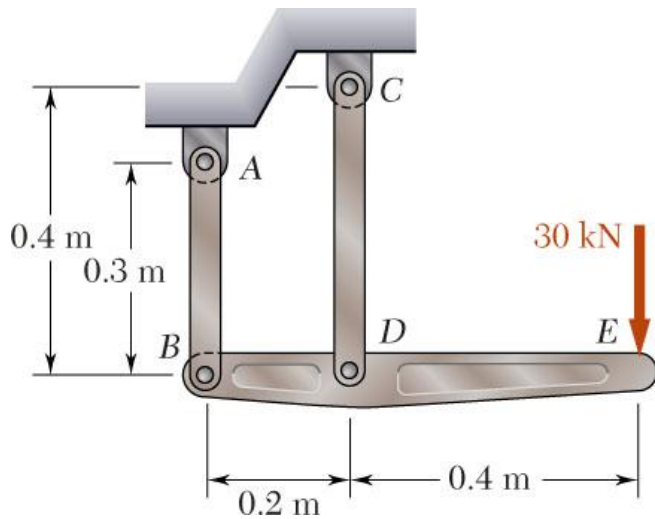
$$\delta_D = \frac{PL}{AE}$$

$$= \frac{(90 \times 10^3 \text{ N})(0.4 \text{ m})}{(600 \times 10^{-6} \text{ m}^2)(200 \times 10^9 \text{ Pa})}$$

$$= 300 \times 10^{-6} \text{ m}$$

$$\delta_D = 0.300 \text{ mm} \downarrow$$

## Sample Problem 2.1



Displacement of D:

$$\frac{BB'}{DD'} = \frac{BH}{HD}$$

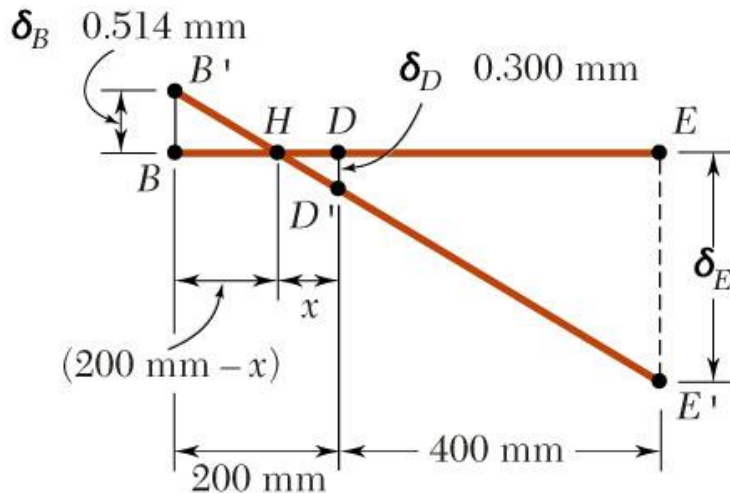
$$\frac{0.514 \text{ mm}}{0.300 \text{ mm}} = \frac{(200 \text{ mm}) - x}{x}$$

$$x = 73.7 \text{ mm}$$

$$\frac{EE'}{DD'} = \frac{HE}{HD}$$

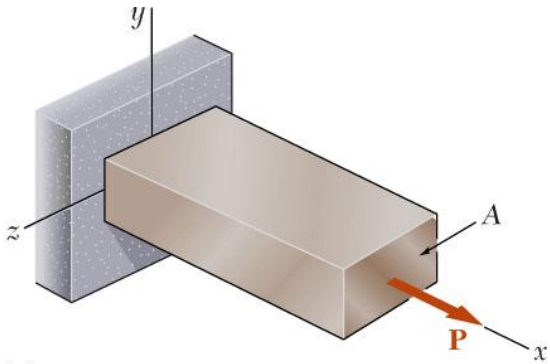
$$\frac{\delta_E}{0.300 \text{ mm}} = \frac{(400 + 73.7) \text{ mm}}{73.7 \text{ mm}}$$

$$\delta_E = 1.928 \text{ mm}$$

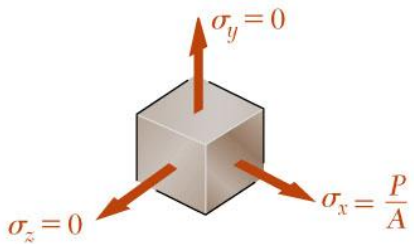


$$\delta_E = 1.928 \text{ mm} \downarrow$$

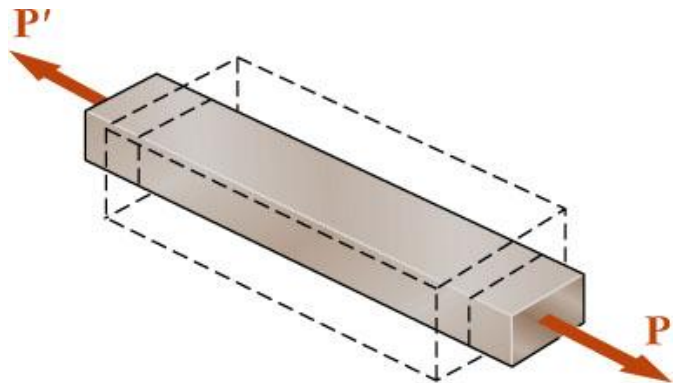
## Poisson's Ratio



(a)



(b)



- For a slender bar subjected to axial loading:

$$\varepsilon_x = \frac{\sigma_x}{E} \quad \sigma_y = \sigma_z = 0$$

- The elongation in the x-direction is accompanied by a contraction in the other directions. Assuming that the material is isotropic (no directional dependence),

$$\varepsilon_y = \varepsilon_z \neq 0$$

- Poisson's ratio is defined as

$$\nu = \frac{\left| \text{lateral strain} \right|}{\text{axial strain}} = -\frac{\varepsilon_y}{\varepsilon_x} = -\frac{\varepsilon_z}{\varepsilon_x}$$

## Shearing Strain

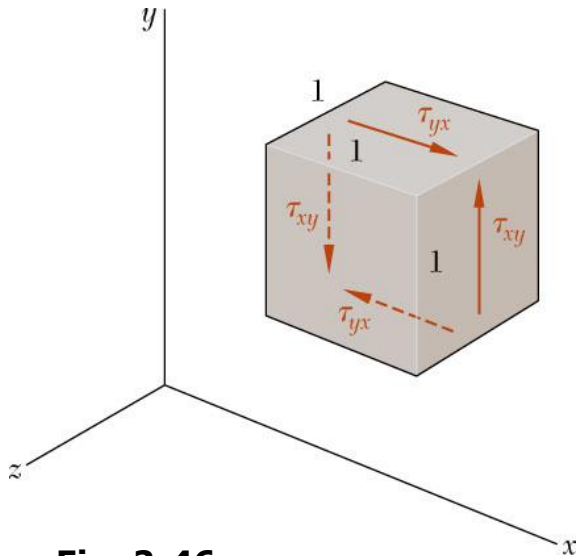


Fig. 2-46

- A cubic element subjected to a shear stress will deform into a rhomboid. The corresponding *shear strain* is quantified in terms of the change in angle between the sides,

$$\tau_{xy} = f(\gamma_{xy})$$

- A plot of shear stress vs. shear strain is similar to the previous plots of normal stress vs. normal strain except that the strength values are approximately half. For small strains,

$$\tau_{xy} = G\gamma_{xy} \quad \tau_{yz} = G\gamma_{yz} \quad \tau_{zx} = G\gamma_{zx}$$

where  $G$  is the modulus of rigidity or shear modulus.

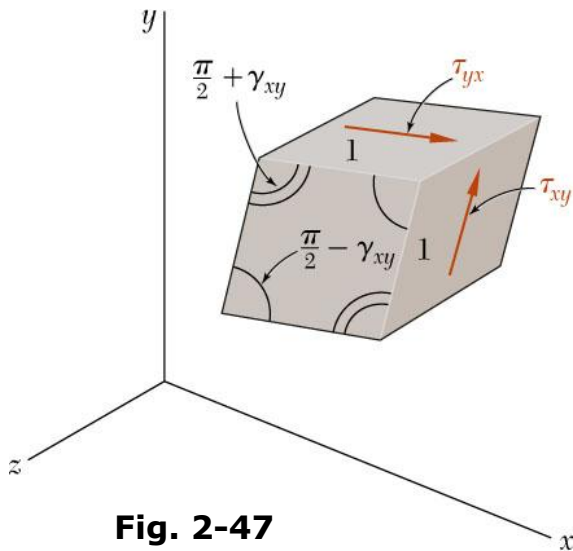
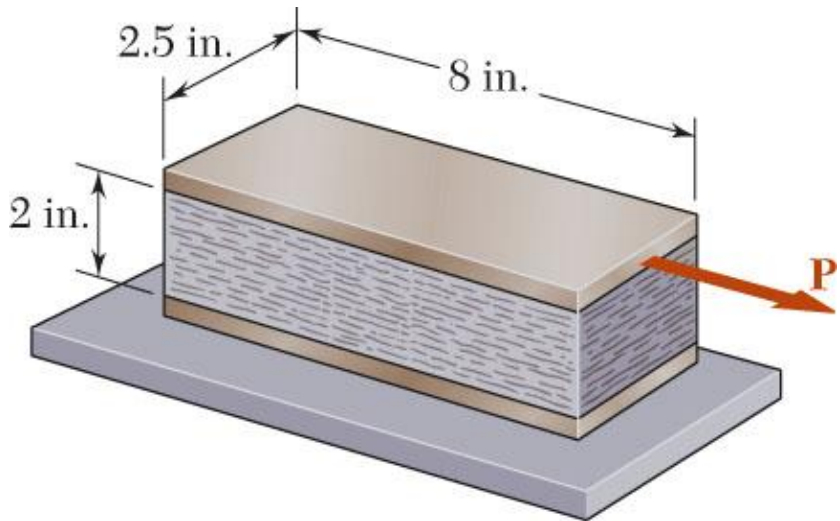


Fig. 2-47

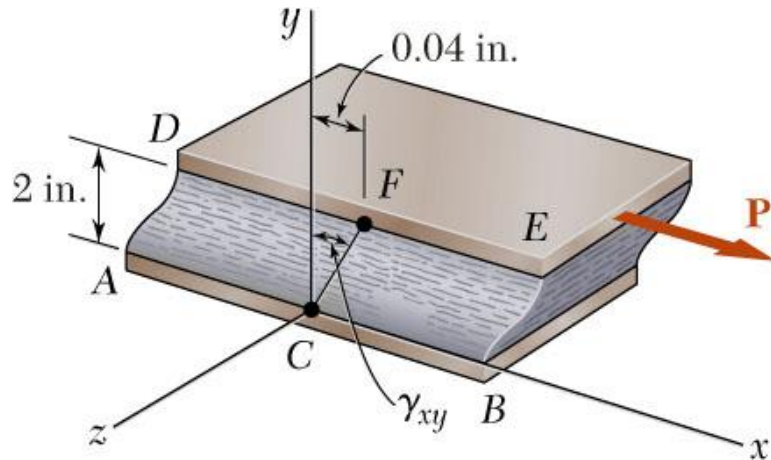
## Example 2.10



A rectangular block of material with modulus of rigidity  $G = 90$  ksi is bonded to two rigid horizontal plates. The lower plate is fixed, while the upper plate is subjected to a horizontal force  $P$ . Knowing that the upper plate moves through 0.04 in. under the action of the force, determine a) the average shearing strain in the material, and b) the force  $P$  exerted on the plate.

### SOLUTION:

- Determine the average angular deformation or shearing strain of the block.
- Apply Hooke's law for shearing stress and strain to find the corresponding shearing stress.
- Use the definition of shearing stress to find the force  $P$ .



- Determine the average angular deformation or shearing strain of the block.

$$\gamma_{xy} \approx \tan \gamma_{xy} = \frac{0.04 \text{ in.}}{2 \text{ in.}} \quad \gamma_{xy} = 0.020 \text{ rad}$$

- Apply Hooke's law for shearing stress and strain to find the corresponding shearing stress.

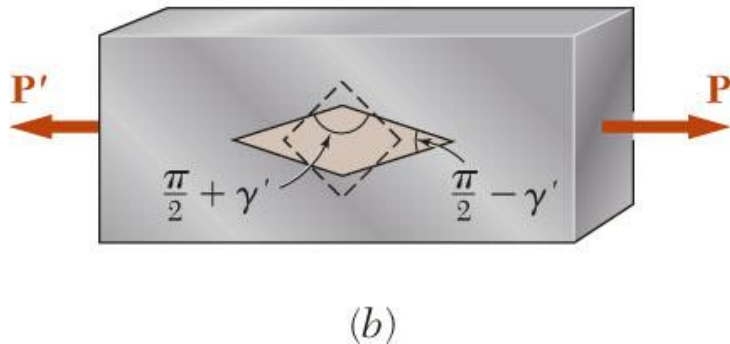
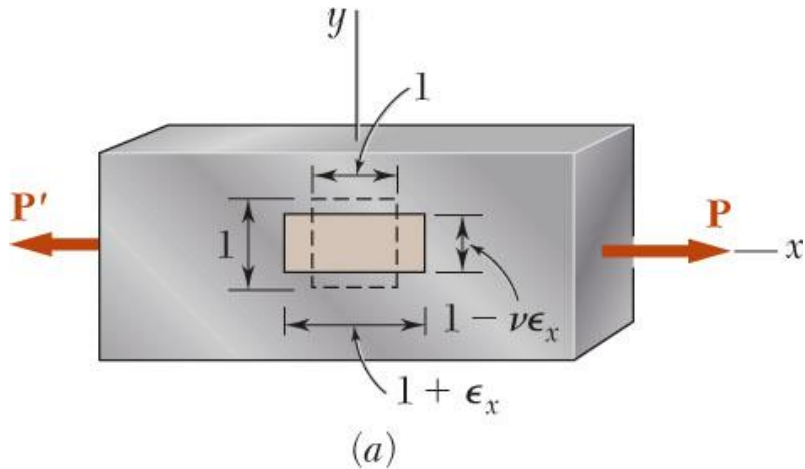
$$\tau_{xy} = G\gamma_{xy} = (90 \times 10^3 \text{ psi})(0.020 \text{ rad}) = 1800 \text{ psi}$$

- Use the definition of shearing stress to find the force  $P$ .

$$P = \tau_{xy}A = (1800 \text{ psi})(8 \text{ in.})(2.5 \text{ in.}) = 36 \times 10^3 \text{ lb}$$

$$P = 36.0 \text{ kips}$$

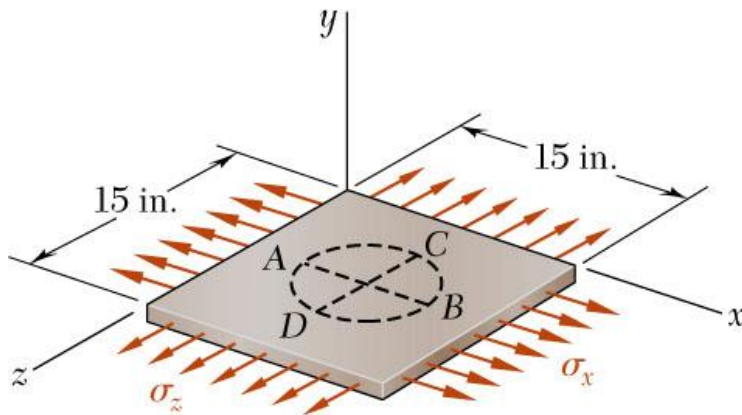
## Relation Among $E$ , $\nu$ , and $G$



- An axially loaded slender bar will elongate in the axial direction and contract in the transverse directions.
- An initially cubic element oriented as in top figure will deform into a rectangular parallelepiped. The axial load produces a normal strain.
- If the cubic element is oriented as in the bottom figure, it will deform into a rhombus. Axial load also results in a shear strain.
- Components of normal and shear strain are related,

$$\frac{E}{2G} = (1 + \nu)$$

## Sample Problem 2.5



A circle of diameter  $d = 9$  in. is scribed on an unstressed aluminum plate of thickness  $t = 3/4$  in. Forces acting in the plane of the plate later cause normal stresses  $\sigma_x = 12$  ksi and  $\sigma_z = 20$  ksi.

For  $E = 10 \times 10^6$  psi and  $\nu = 1/3$ , determine the change in:

- the length of diameter  $AB$ ,
- the length of diameter  $CD$ ,
- the thickness of the plate, and
- the volume of the plate.

## SOLUTION:

- Apply the generalized Hooke's Law to find the three components of normal strain.
- Evaluate the deformation components.

$$\begin{aligned}\varepsilon_x &= +\frac{\sigma_x}{E} - \frac{\nu\sigma_y}{E} - \frac{\nu\sigma_z}{E} \\ &= \frac{1}{10 \times 10^6 \text{ psi}} \left[ (12 \text{ ksi}) - 0 - \frac{1}{3}(20 \text{ ksi}) \right] \\ &= +0.533 \times 10^{-3} \text{ in./in.}\end{aligned}$$

$$\begin{aligned}\varepsilon_y &= -\frac{\nu\sigma_x}{E} + \frac{\sigma_y}{E} - \frac{\nu\sigma_z}{E} \\ &= -1.067 \times 10^{-3} \text{ in./in.}\end{aligned}$$

$$\begin{aligned}\varepsilon_z &= -\frac{\nu\sigma_x}{E} - \frac{\nu\sigma_y}{E} + \frac{\sigma_z}{E} \\ &= +1.600 \times 10^{-3} \text{ in./in.}\end{aligned}$$

$$\delta_{B/A} = \varepsilon_x d = (+0.533 \times 10^{-3} \text{ in./in.})(9 \text{ in.})$$

$$\delta_{B/A} = +4.8 \times 10^{-3} \text{ in.}$$

$$\delta_{C/D} = \varepsilon_z d = (+1.600 \times 10^{-3} \text{ in./in.})(9 \text{ in.})$$

$$\delta_{C/D} = +14.4 \times 10^{-3} \text{ in.}$$

$$\delta_t = \varepsilon_y t = (-1.067 \times 10^{-3} \text{ in./in.})(0.75 \text{ in.})$$

$$\delta_t = -0.800 \times 10^{-3} \text{ in.}$$

- Find the change in volume

$$e = \varepsilon_x + \varepsilon_y + \varepsilon_z = 1.067 \times 10^{-3} \text{ in}^3/\text{in}^3$$

$$\Delta V = eV = 1.067 \times 10^{-3} (15 \times 15 \times 0.75) \text{ in}^3$$

$$\Delta V = +0.187 \text{ in}^3$$